

Structural Inspiration for Building Mid-Rise with Wood.

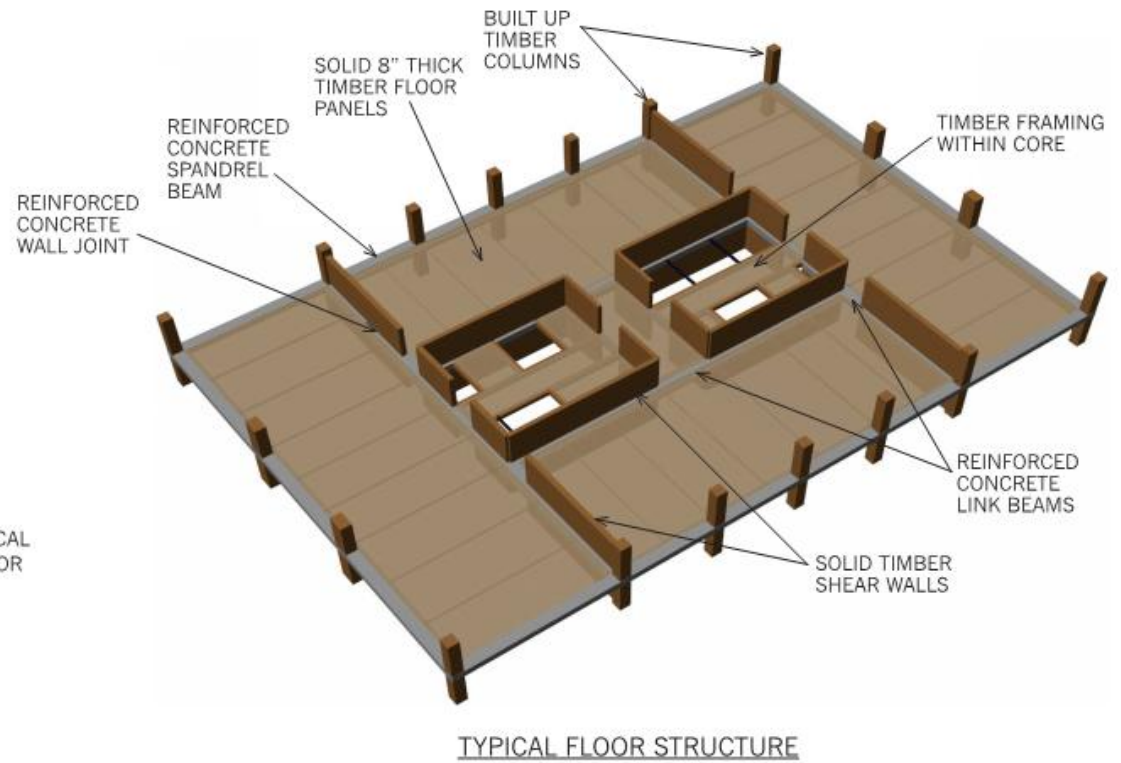
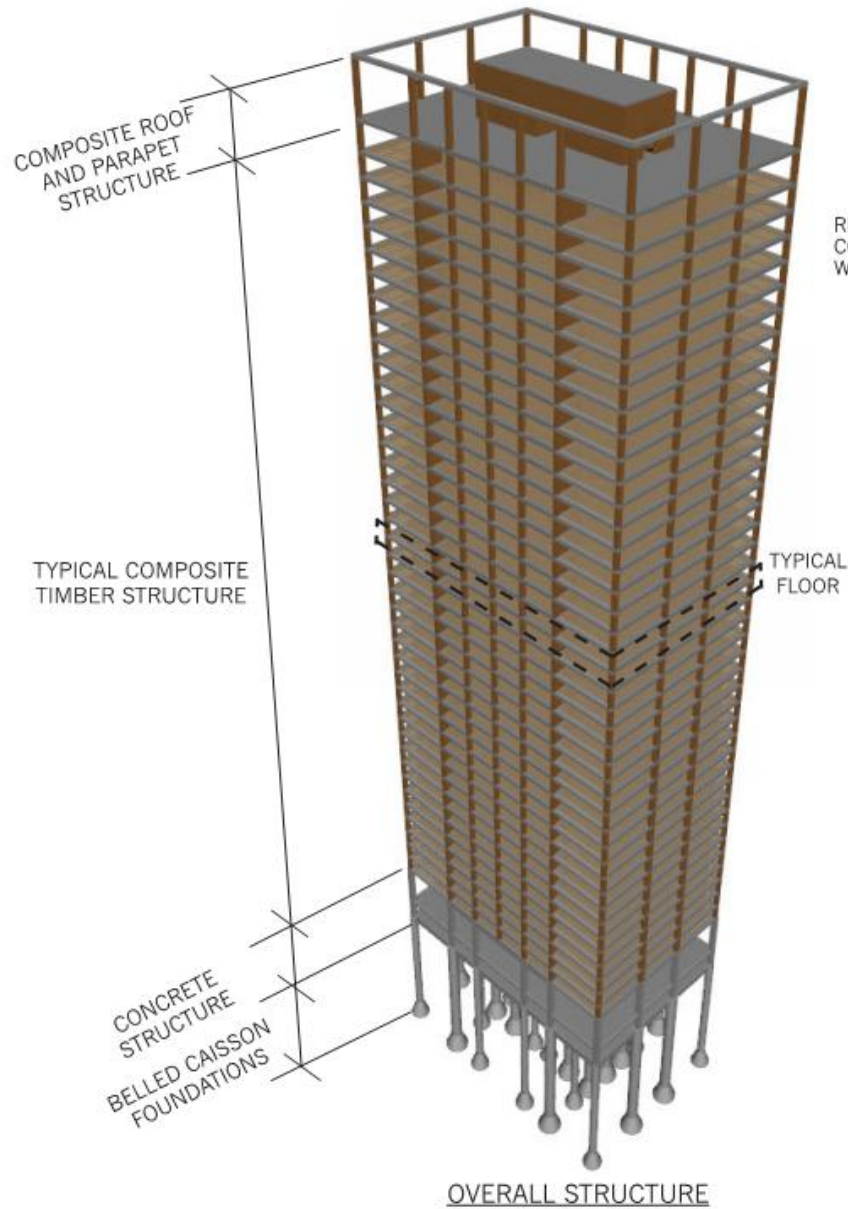
Matthew Reid, MASc., P.Eng



Read Jones Christoffersen
Consulting Engineers

What is Mid-Rise?

(5 to 11 stories)



STRUCTURAL SYSTEM DESCRIPTION

THE STRUCTURAL SYSTEM PROPOSED IS THE CONCRETE JOINTED TIMBER FRAME. THIS SYSTEM CONSISTS OF SOLID MASS TIMBER PRODUCTS FOR THE PRIMARY MEMBERS SUCH AS THE FLOOR PANELS, COLUMNS, AND SHEAR WALLS. THE PRIMARY MEMBERS ARE CONNECTED WITH STEEL REINFORCING THROUGH CONCRETE JOINTS.

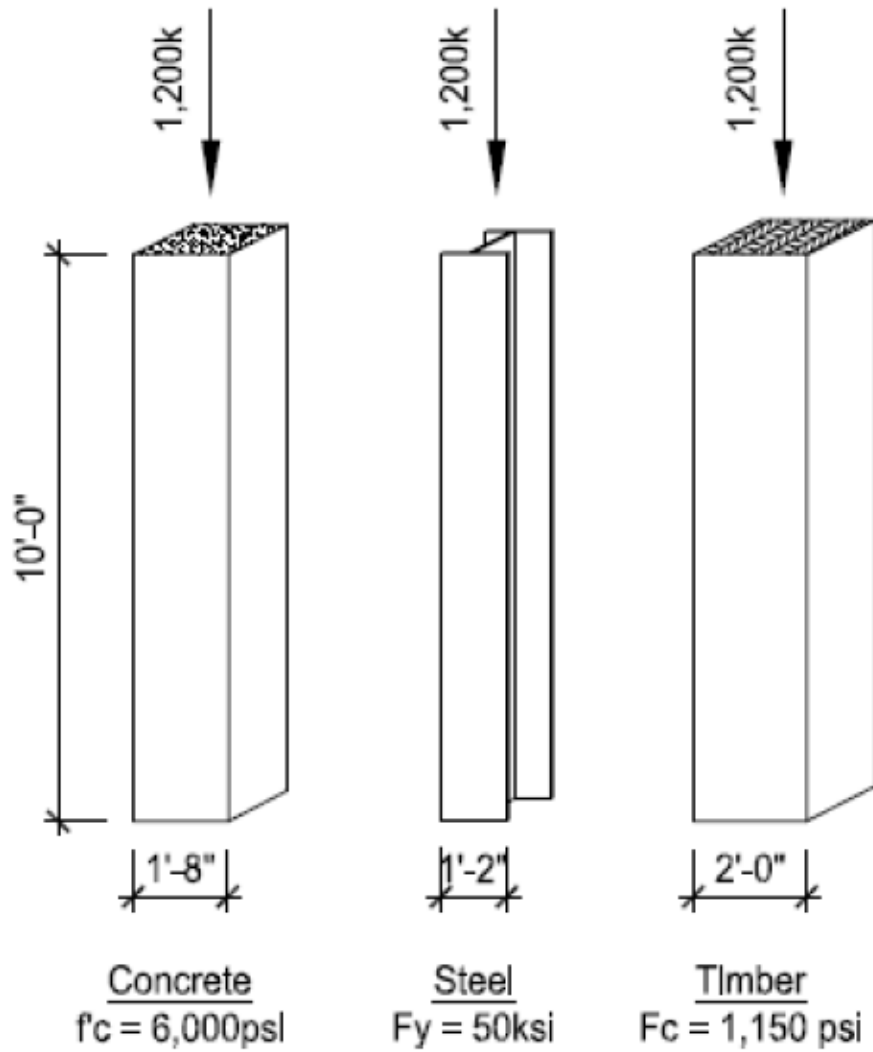
THE GRAVITY FRAMING SYSTEM UTILIZES SOLID 8" THICK TIMBER FLOOR PANELS WHICH SPAN BETWEEN THE SHEAR WALL CORE AT THE CENTER OF THE BUILDING AND COLUMNS AT THE PERIMETER OF THE BUILDING. THE ENDS OF THE FLOOR PANELS ARE RESTRAINED FROM ROTATING BY THE CONCRETE JOINTS AND VERTICAL STRUCTURE. THIS CONNECTION SCHEME ALLOWS THE FLOOR SYSTEM TO BE MORE ECONOMICAL.

THE LATERAL LOAD RESISTING SYSTEM UTILIZES SOLID TIMBER SHEAR WALLS WHICH ARE COUPLED WITH REINFORCED CONCRETE LINK BEAMS. THE SYSTEM IS DESIGNED TO BE EFFICIENT IN RESISTING NET UPLIFT DUE TO WIND OVERTURNING.

ESTIMATED QUANTITIES

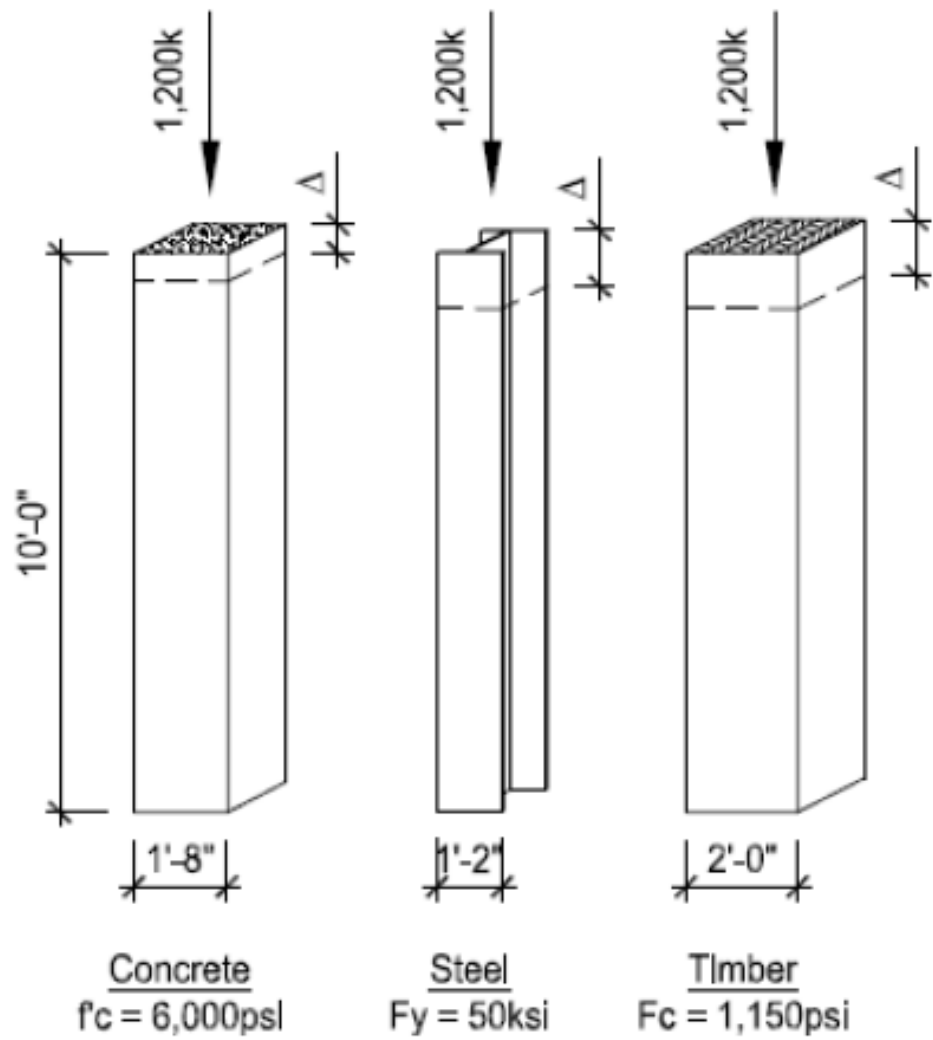
SUB AND SUPERSTRUCTURE:
 TIMBER: 0.80 cu.ft/sf
 CONCRETE: 0.25 cu.ft/sf
 REINFORCEMENT: 1.7 psf
 STRUCTURAL STEEL: 0.3 psf

FOUNDATIONS:
 CONCRETE: 0.09 cu.ft/sf
 REINFORCEMENT: 0.1 psf



Material Axial Strength Comparison

Material	Reinforced Concrete	Steel	Timber
Cross Section	20"x20"	W14x99	24"x24"



Material Axial Stiffness Comparison

Material	Concrete	Steel	Timber
Cross Section	20"x20"	W14x99	24"x24"
Axial Stiffness	15,600 k/in	7,000 k/in	6,700 k/in
Movement, Δ	0.08"	0.17"	0.18"

Figure A.2: Material Axial Stiffness



Wall panel anchored to floor panel.



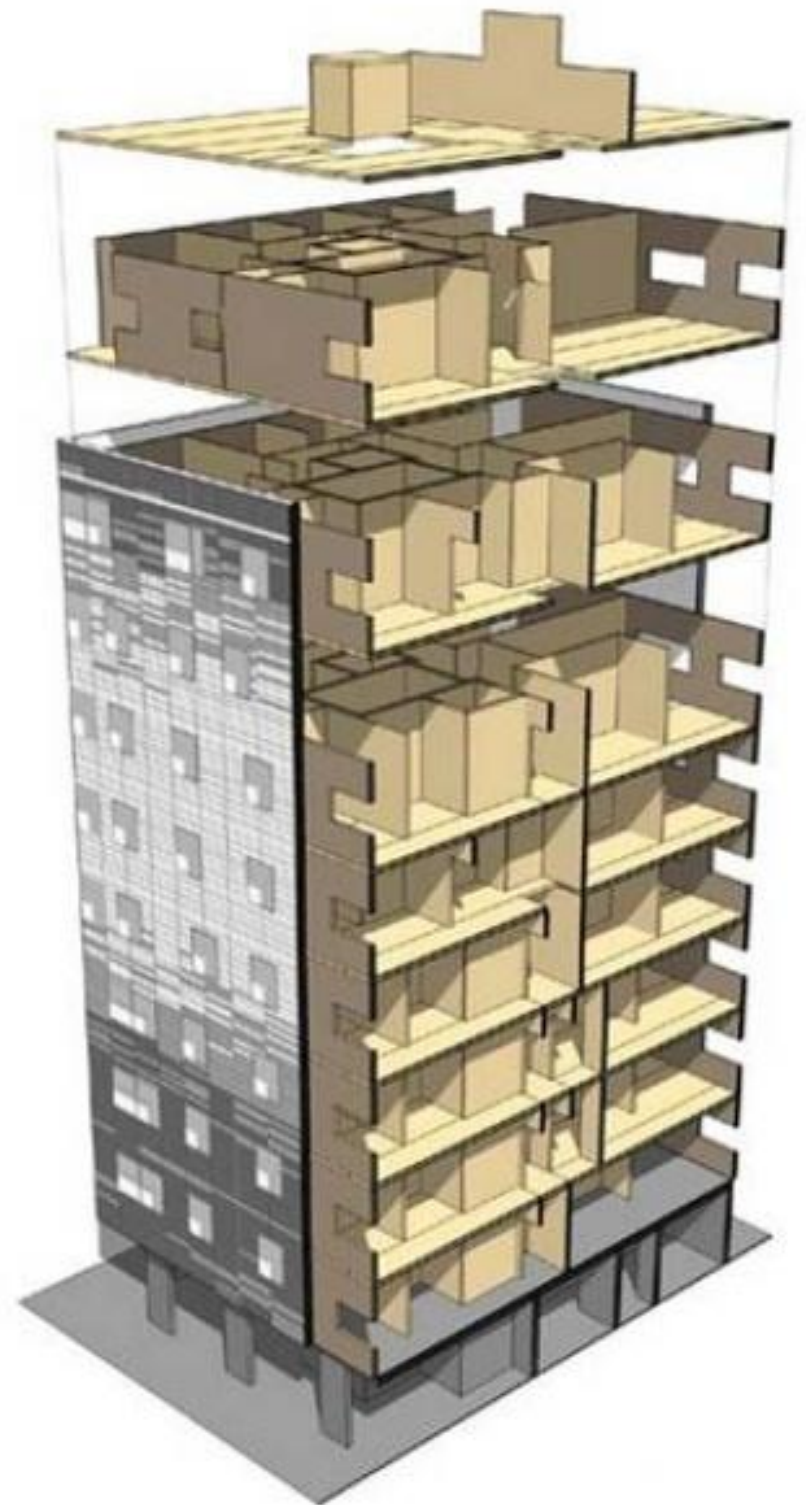
Typical internal wall arrangement.



Floor panel installation.



Floor panel installation.







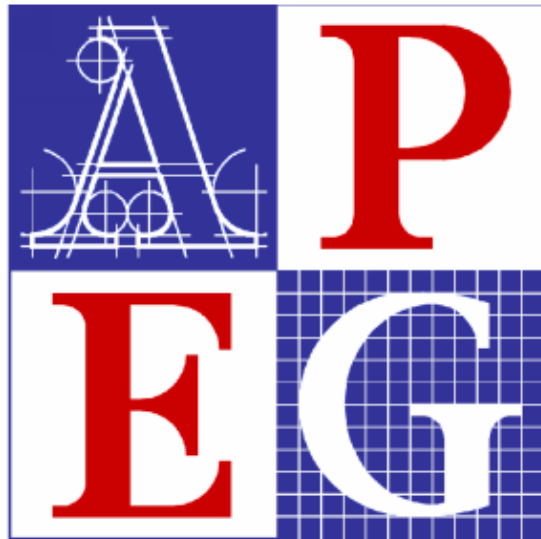


What Use?

Condos, Office

Mixed Use w/ Retail

APEGBC Technical and Practice Bulletin



Professional Engineers
and Geoscientists of BC

w w w . a p e g . b c . c a

Structural, Fire Protection and Building Envelope Professional Engineering Services for 5 and 6 Storey Wood Frame Residential Building Projects (Mid-Rise Buildings)

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Revised March 1, 2011



Technical Guide for the Design and Construction of Tall Wood Buildings in Canada

Special Publication SP-55E

Edited by
Erol Karacabeyli P.Eng.
Conroy Lum P.Eng.

2014 – First Edition

Inspiration for using Wood!

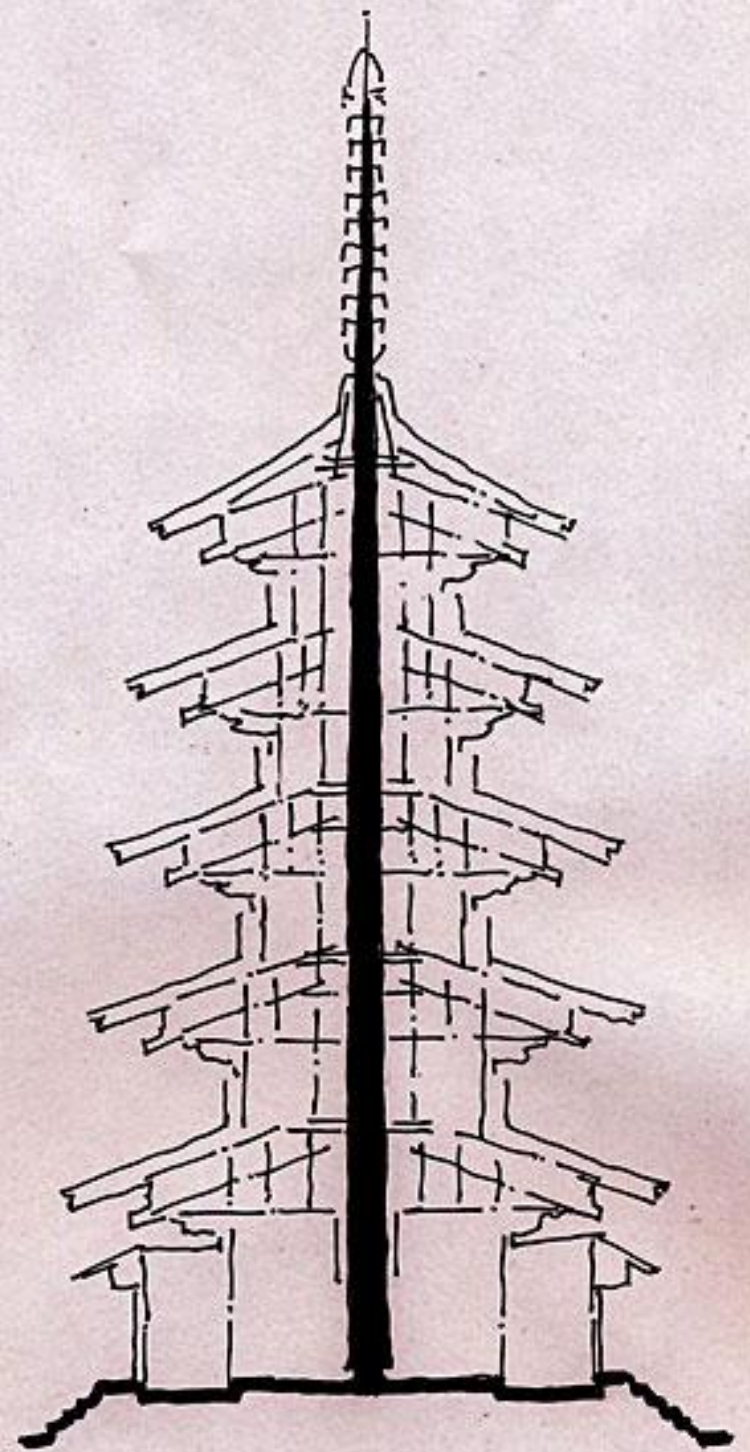
What's your story?

The Japanese Pagoda

Central Core



願林寺三重塔（兵庫県加古川市）；全体
撮影日：1999年12月4日







Who are you?

(Architect, Developer, Contractor, Engineer)

Century Old Lessons

Learning from 4-story Condos

An Urban Office

Framing Types

What are you used to?

Light Framing

Light Steel Studs



Light Wood Framing



Light Framing

Light Steel Studs

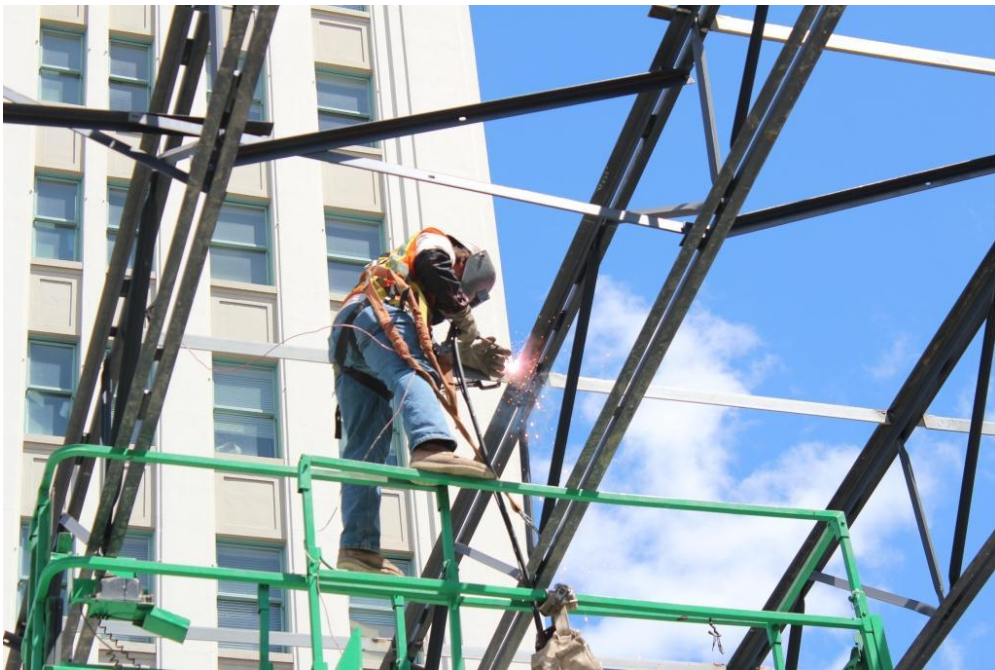


Light Wood Framing



Light to Heavy

OWSJ



Wood I-Joists



Heavy Framing

Structural Steel

- I-Beams
- HSS Tubes
- Channels



Heavy Timber

- Glulam
- Parallel Strand Lumber
- Solid Sawn



Heavy Framing

Structural Steel



Heavy Timber



Glulam

Species

D.Fir (from BC)

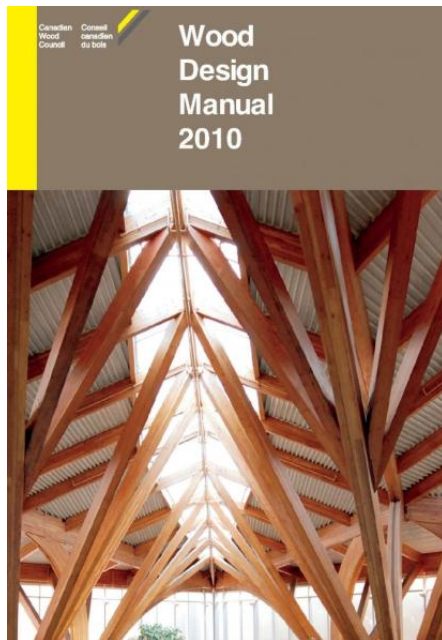
Spruce-Pine (Quebec)

Nordic (Black Spruce)

Sizes

80, 130 – 365 (3 1/8" – 14")

Laminations (38mm 1.5")



Slabs (Mass Construction)

Concrete

- CIP Slabs
- Precast Slabs
- Walls

Mass Timber

- CLT – Cross Laminated
- LSL – Laminated Strand
- LVL – Laminated Veneer



Slabs (Mass Construction)

Concrete



Mass Timber



CLT

Nordic

78, 105, 131, 175, 220, 244, 314

175 unprotected – FRR 96 min.

175 w/ GWB – FRR 124 min

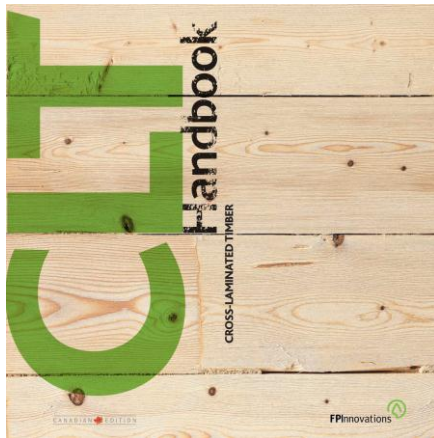
245 unprotected – FRR 178 min.

Structurlam

99, 169, 239, 309

Vibration controlled span

3.5m, 4.9m, 6.2m, 7.4m.



Basic Framing Types

LIGHT WOOD FRAMING

< 3" (75mm)

PLYWOOD / T&G

JOISTS

THIN

(SAWN, I-JOIST, LVL, LSL)

BEAMS

STUDS

HEAVY TIMBER FRAMING

> 4" / 6" (100mm/150mm)

BEAMS

COLUMNS

(SAWN, GLULAM, PSL)

MASS TIMBER

SOLID PLANKS

DECKING

WALLS

(SAWN, LSL, LVL, CLT)

HYBRID CONSTRUCTION

w/

STEEL, CONCRETE OR MASONRY

Comparison of Wood Products

Table 1 Comparison of Various Wood Products

	Dim. lumber No.1/No.2 stud, SPF	Dim. lumber SS, D.Fir.	Glulam douglas fir 24FE	LVL 2.0E	LSL 1.55E	PSL D.Fir 2.2E	CLT wall 5-Ply
Compression stress parallel to grain, f_c (MPa)	11.5	19.0	30.2	35.2	22.5	31.9	11.6 (long) 5.4 (trans) ¹
Compression stress perpendicular to grain, f_{cp} (MPa)	5.3	7.0	7	9.4 (beam) 6.9 (plank)	10.0 (beam) 6.1 (plank)	9.4 (beam) 6.0 (plank)	5.3 ²
Longitudinal shear, f_v (MPa)	1.5	1.9	2	3.7 (beam) 1.8 (plank)	5.2 (beam) 2.0 (plank)	3.7 (beam) 2.7 (plank)	2.2 ³
Bending at extreme fibre, f_b (MPa)	11.8	16.5	30.6	37.6 (beam) 37.6 (plank)	29.6 (beam) 33.3 (plank)	37.0 (beam) 35.7 (plank)	N/A ⁴

¹ The longitudinal plies are assumed to have f_c of 30 MPa (10ksi) (SS lumber), and the transverse plies are assumed to have f_c of

Fire Ratings
&
Architectural Coordination

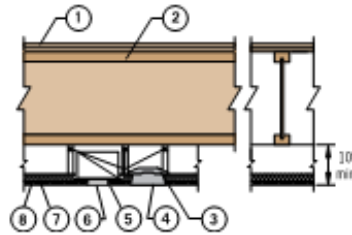
ONE-HOUR FLOOR/CEILING, ROOF/CEILING ASSEMBLIES

Lightweight concrete or approved gypsum concrete topping with appropriate sheathing can be substituted for the decking material shown in any of these assemblies.

Assembly A

Intertek
WNR FCA 60-11
ICC-ES ESR-1153

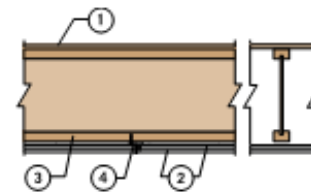
1. Double wood floor
2. TJI® joist with minimum 1½" flange depth (see TJI® Joist Specifications table on page 16 for flange sizes)
3. Fixture protection
4. 24" x 48" recessed light fixture
5. Cold-rolled channels
6. 12" air diffuser
7. Minimum 1"-thick (4 pcf minimum) mineral wool blankets
8. ½" acoustical panels, 24" x 24" or 24" x 48", supported by an approved exposed fire-rated suspension system



Assembly B

Intertek
WNR FCA 60-01
WNR FCA 60-03
PFS FA-1
ICC-ES ESR-1153

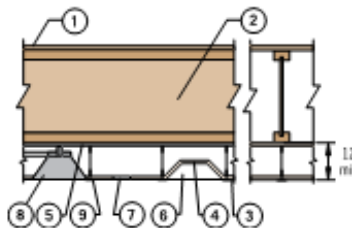
1. 48/24 tongue-and-groove, span-rated sheathing (Exposure 1)
 2. Two layers ½" Type X gypsum board
 3. TJI® joist
 4. Resilient channels (optional)*
- Optional.** Minimum 3½"-thick glass fiber insulation or non-combustible insulation, rated R-30 or less.*
- *Resilient channels are required when insulation is used.



Assembly C

ICC-ES ESR-1153

1. 48/24 tongue-and-groove, span-rated sheathing (Exposure 1)
2. TJI® joist
3. ½" ceiling panels, 24" x 24"
4. Fixture protection
5. ½" Type C gypsum board or ½" Type X gypsum board
6. 24" x 48" recessed light fixture
7. 6" x 12" opening for return air
8. 12" diameter diffuser opening
9. Steel suspension grid



Note: Additional restrictions apply to Intertek's version of this listing. Refer to WNR FCA 60-08 for additional information.

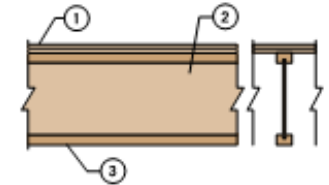
ONE-HOUR FLOOR/CEILING, ROOF/CEILING ASSEMBLIES

Lightweight concrete or approved gypsum concrete topping with appropriate sheathing can be substituted for the decking material shown in any of these assemblies.

Assembly E

ICC-ES ESR-1153

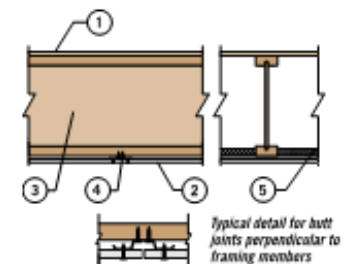
1. Double wood floor or single layer of 48/24 tongue-and-groove span-rated sheathing (Exposure 1)
2. TJI® joist
3. An approved ceiling system that will provide a 40-minute finish rating



Assembly F

Intertek
WNR FCA 60-07
ICC-ES ESR-1153

1. 48/24 tongue-and-groove, span-rated sheathing (Exposure 1)
2. ½" Type C gypsum board
3. TJI® joist with a minimum depth of 1½" and a minimum flange size of 1½" thick x 3½" wide (see TJI® Joist Specifications table on page 16 for flange sizes). Joists spaced at 24" on-center, maximum.
4. Resilient channel at 16" on-center
5. Minimum 1½"-thick (2.5 pcf minimum) mineral wool batts



TWO-HOUR FLOOR/CEILING, ROOF/CEILING ASSEMBLY

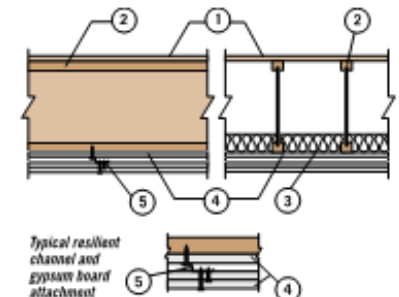
Lightweight concrete or approved gypsum concrete topping with appropriate sheathing can be substituted for the decking material shown in any of these assemblies.

Assembly G

Intertek
WNR FCA 120-3
ICC-ES ESR-1153

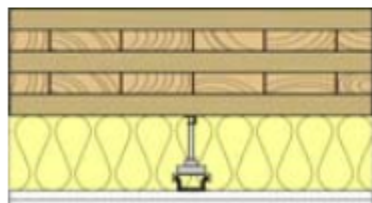
Assembly G is typically used for garage/living unit separation

1. 48/24 tongue-and-groove, span-rated sheathing (Exposure 1)
2. TJI® joist, 24" on-center maximum
3. Optional glass fiber insulation, unfaced batts, 3½" thick in plenum, supported by stay wires 12" on-center and centered on joist bottom flanges
4. Three layers of ½" Type C gypsum board
5. Resilient channels at 16" on-center located between first and second layers of gypsum board

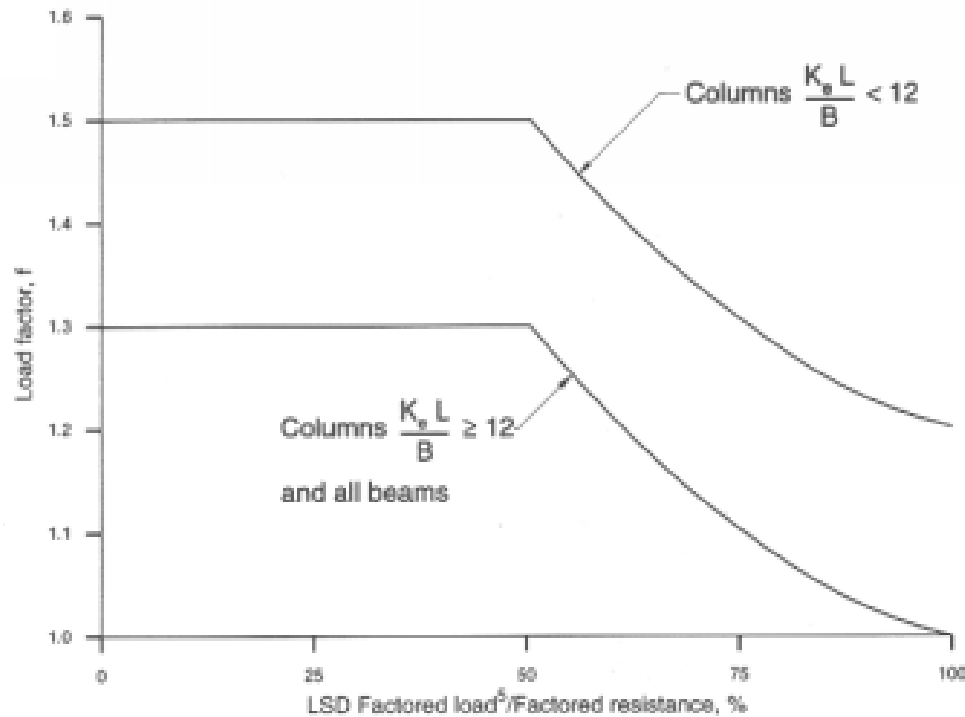


NORDIC'S CLT FRR

P2	<ul style="list-style-type: none"> - carpet or floating flooring, about 2/5 in. - resilient underlayment (rubber mat or textured felt), 0.12 in. - at least 15.6 lb/ft² wet topping (concrete, gypcrete, gypsum) - resilient underlayment (2/5 in. rubber mat, 3/4 in. textured felt, or 1/2 in. low density wood fiberboard) - cross-laminated timber 6-7/8 in. 	1,5 h	> 53 ⁽⁴⁾
P2.1	+ 1 layer 5/8 in. Type X gypsum board	2 h	> 53 ⁽⁴⁾
P3	<ul style="list-style-type: none"> - carpet or floating flooring, about 2/5 in. - resilient underlayment (rubber mat or textured felt), 0.12 in. - at least 25 kg/m² dry topping (20 mm Fermacell, cement fibreboard, or Fibrerock) - resilient underlayment (2/5 in. rubber mat, 3/4 in. textured felt, or 1/2 in. low density wood fiberboard) - cross-laminated timber 6-7/8 in. 	1,5 h	> 48 ⁽⁴⁾
P3.1	+ 1 layer 5/8 in. Type X gypsum board	2 h	> 48 ⁽⁴⁾
P4	- cross-laminated timber 6-7/8 in.	1,5 h	39 ⁽⁵⁾
P4.1	<ul style="list-style-type: none"> - cross-laminated timber 6-7/8 in. - sound insulation clips of 4 in. high - metal hat channels, at min. 16 in. o.c. - sound insulation material, 4 in. - 2 layers 1/2 in. Type X gypsum board 	2 h	64



Fire Rating of Glulam



1hr FRR

Beam – 175x304

Column – 315 x 304

$$FRR = 0.1fB \left[4 - 2\frac{B}{D} \right]$$

for beams exposed to fire on 4 sides,

$$FRR = 0.1fB \left[4 - \frac{B}{D} \right]$$

for beams exposed to fire on 3 sides,

$$FRR = 0.1fB \left[3 - \frac{B}{D} \right]$$

for columns exposed to fire on 4 sides,

$$FRR = 0.1fB \left[3 - \frac{B}{2D} \right]$$

for columns exposed to fire on 3 sides,

The case of Timber vs Steel for fire performance.

During a fire a room temperature rises from
70 °F (21 °C) to 1100 °F (600 °C).

The Brentwood Story

The case of a 0 hr Fire Rated Roof.



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Etobicoke

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PUBLIC
LIBRARY

NO PARKING
ANYTIME

NO PARKING
ANYTIME







TRICKLE WAY

DANGER
DUE TO

NO PARKING
ANYTIME

ALL PERSONNEL
ENTERING SITE MUST
REPORT TO SITE OFFICE

ATTENTION
CONSTRUCTION
DELIVERIES AND
CONSTRUCTION
PERSONEL ONLY



Inspiration from Historic Mid-Rise Buildings

A case study in Vancouver & Toronto

In Toronto

- 128 Buildings: 2 – 8 Storeys
- 43 Buildings: 5+
- 19 Buildings: 7+
- Max. Height: 30m (100 ft)
- Max. Floor Space:
 - 20,440 sm (220,000 sf)
- Years of Construction:
 - 1859 to 1941



A Study on Historical Tall-Wood Buildings in Toronto and Vancouver

First edition

Project No. 301006152 Canadian Forest Service Final Report 2012/13

May 2013

Author

Kenneth Koo, P. Eng, P.E., Industry Advisor, Advanced Building Systems

This project was financially supported by the Canadian Forest Service under the Contribution Agreement existing between the Government of Canada and FPIinnovations.



176

VEGANE
GASTRO

VEGANE GASTRO

NO
PARKING
IN FRONT OF
DRIVEWAY
ANY TIME

NO
PARKING
IN FRONT OF
DRIVEWAY
ANY TIME

RENTAL

RENTAL
COMMERCIAL
MAILING
INDUSTRIAL
SCHEDULE

RENTAL
COMMERCIAL
MAILING
INDUSTRIAL
SCHEDULE

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· ARCHITECT · AND ·
· BUILDER ·

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Toronto, Montreal—JULY, 1907—Winnipeg, Vancouver

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\$2.00 PER YEAR.

http://digital.library.mcgill.ca/cab/search/browse_frameset.htm

the advisability of relieving at least to a certain extent the glaring monotony of all white floors and walls.

WAREHOUSE BUILDINGS.

In building a modern warehouse attention must be paid to the three classes of construction, commonly known as "ordinary," "mill" and "slow-burning" construction. All are intended to safely sustain a load of 100 pounds per square foot of floor space, although this capacity may be indefinitely increased as the purposes for which the building is intended vary.

The term "ordinary construction" is defined in the revised Toronto building by-law as meaning "a building with wood joists and wood or iron posts, columns and beams, which are not protected with fire-resisting coverings, but having the external and party walls constructed of brick, stone or some other incombustible material, the roof of such buildings being covered with tin, iron, copper, slate, tile, felt and gravel or other material of an incombustible nature."

On the other hand, "mill construction" is defined as meaning a "building in which all the wooden girders and joists, supporting floors and roof, have a sec-

Where planks are set on edge and are not over two inches thick, with a matched wearing floor above, the tongue and groove or splice may be omitted; in such cases the planks shall be well spiked at intervals of 15 inches with spikes of sufficient length to penetrate two-thirds the thickness of the adjoining plank.

The term "slow-burning construction," according to the by-law, "shall apply to all buildings in which mill construction is used and in which the structural members, which carry the loads and strains which come upon the floors and roofs thereof, are entirely enveloped in incombustible material. This class of building shall have double floors and roof, with one thickness of asbestos paper, weighing not less than 14 pounds each per 100 square feet, between the upper and lower thicknesses of floors and roof. The under side of all floors shall be protected in the same manner as the wood structural members.

Ordinarily the two latter classes of construction are regarded as practically the same, distinction being generally made between the first two only. The joists in all cases have a span of 20 feet from one bearing to the other and in ordinary construction are 3 x 12 inches, spaced 12 inches centre to centre, while those

ORDINARY CONSTRUCTION

- WOOD JOISTS
- FLOORS NOT FRR PROTECTED
- EXTERIOR & PARTYWALLS ARE NON-COMBUSTIBLE.

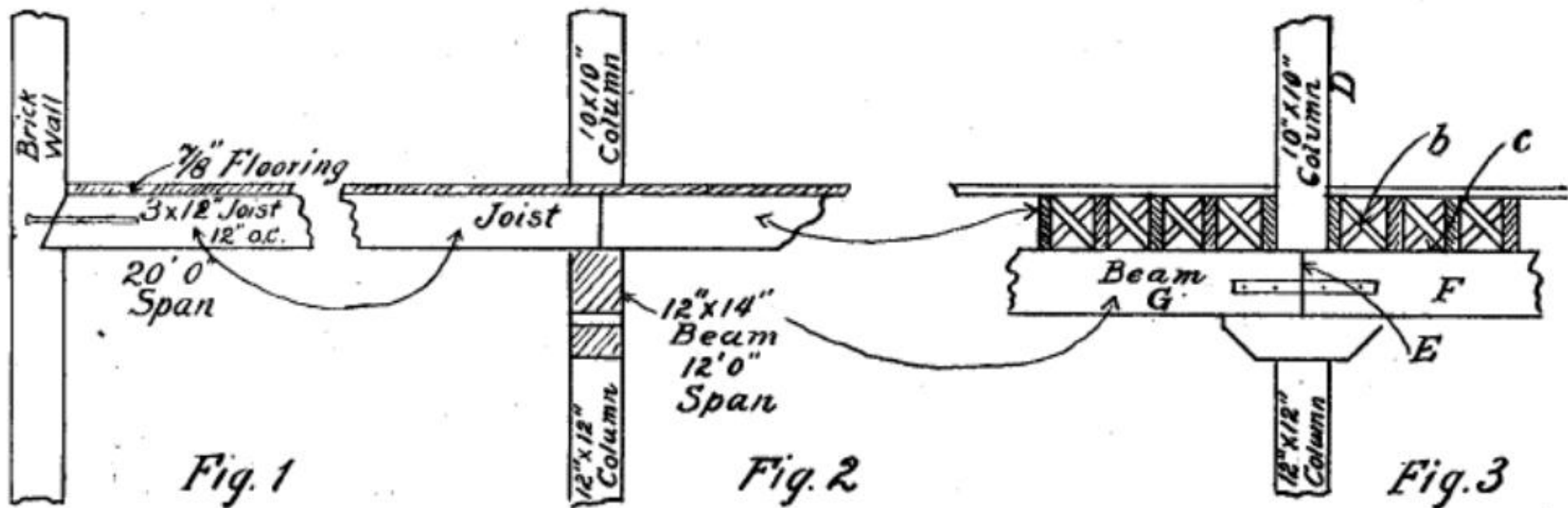
MILL CONSTRUCTION

- WOOD GIRDERS AND JOISTS
- FLOOR > 2" THICK.
- IF NO T&G, WELL SPIKED

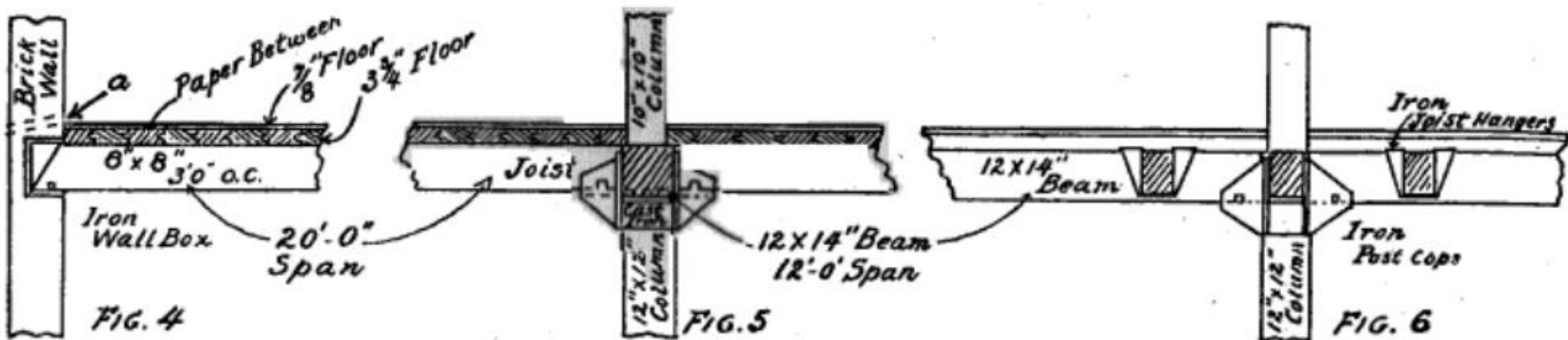
SLOW-BURNING CONSTRUCTION

- SAME AS MILL
- FLOOR & ROOFS ENTIRELY ENVELOPED IN NON-COMBUSTIBLE MATERIAL

Hybrid Construction



ORDINARY CONSTRUCTION.



MILL OR SLOW-BURNING CONSTRUCTION

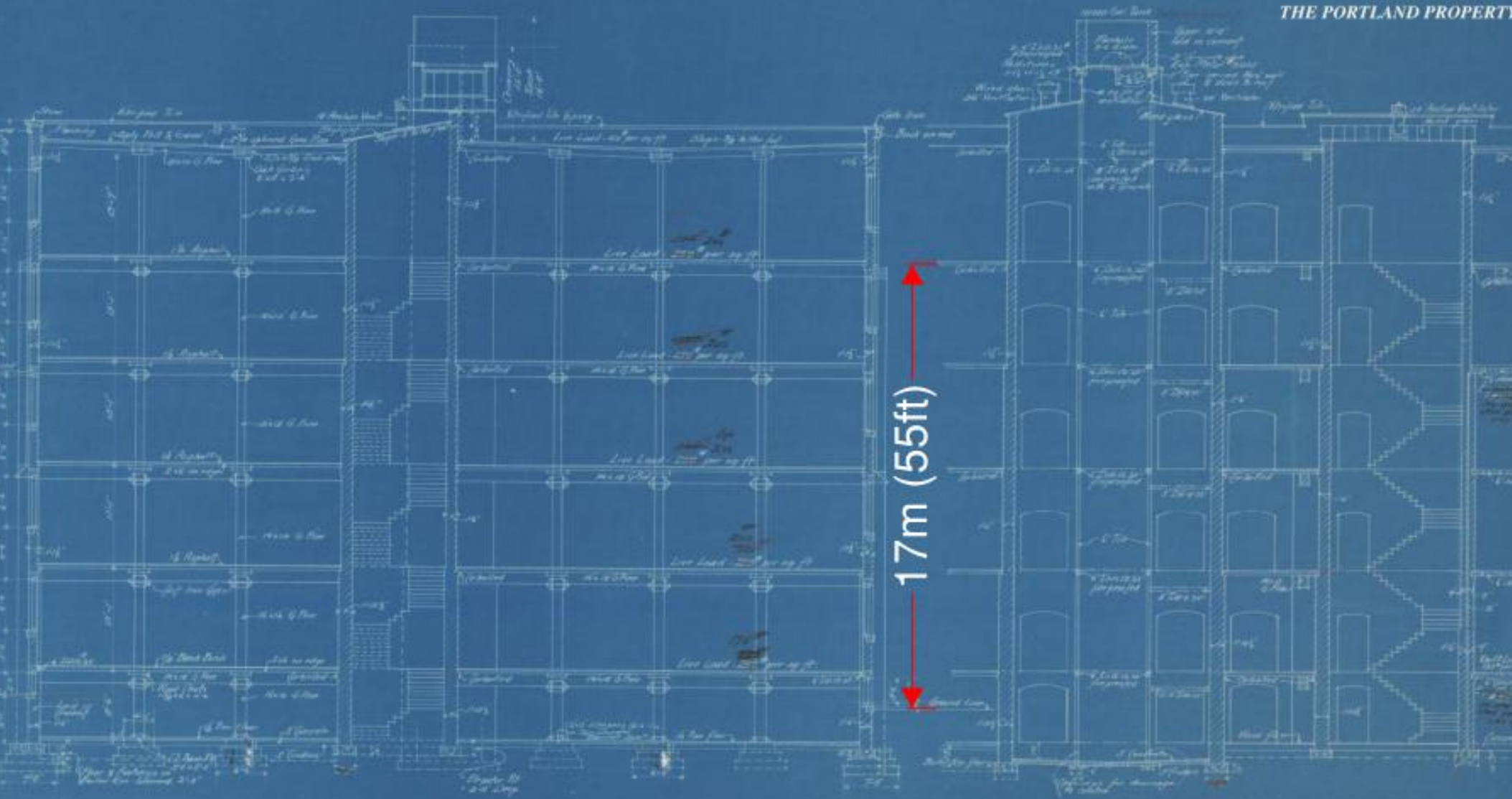




FACTORY FOR THE
 STEELE, BRIGGS, SEED & CO. LIMITED
 COR. OF CADENCE SQ. & SPRADINA AVE
 SCALE - 1/8" = 1'-0"

January 25, 1912

COURTESY OF
 THE PORTLAND PROPERTY



SECTION AA

SECTION BB

Note - See Detail of Base sections

SPROATT & ARCHER
 ENGINEERS
 1000 BROADWAY
 NEW YORK

FACTORY FOR THE
 STEEL, BRIGGS SEED & LIA
 CO. OF CLARENCE, SD. & GRADIN
 SCALE - 1/8" = 1'-0"

*Spratt & Hulpe
 Toronto*

COURTESY OF
 THE PORTLAND PROPERTY GROUP

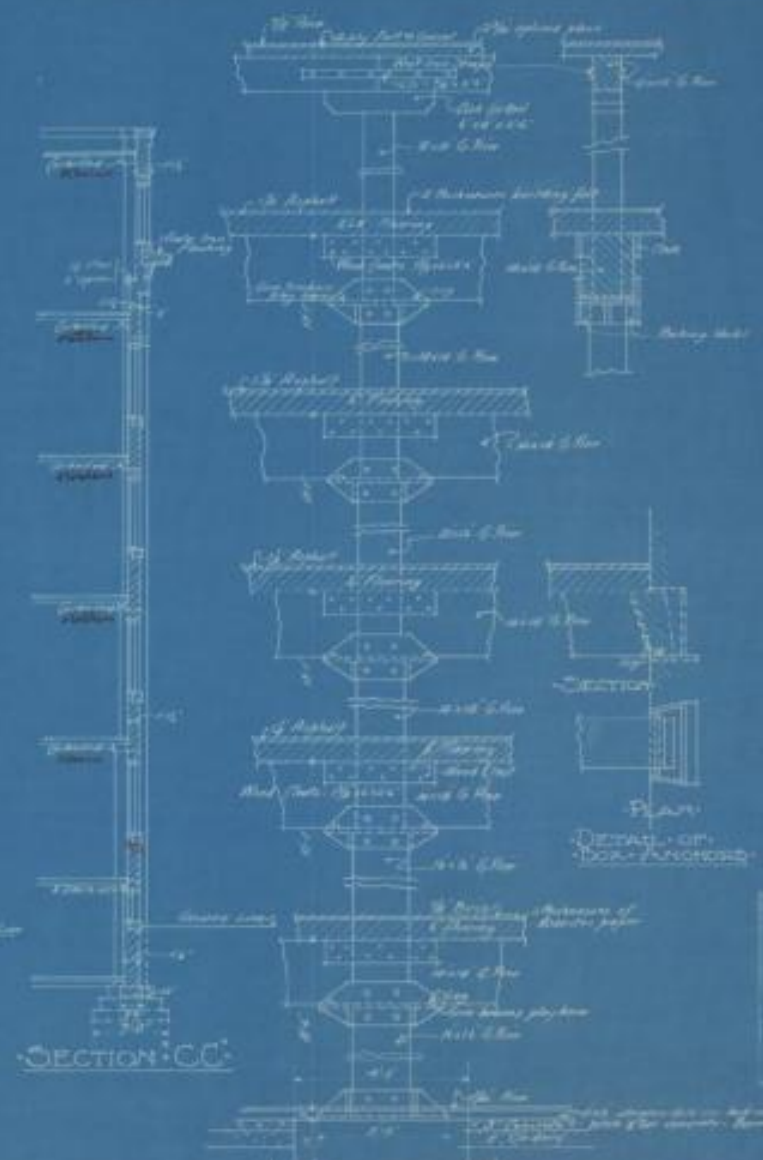


SPRATT & HULPE
 ARCHITECTS,
 TORONTO
 CONTRACT No. 222
 SHEET No. 2
 DATE Aug. 11, 1914

FIRST FLOOR PLAN

FACTORY FOR THE
 STEELE, DRIGGS & SEED CO. LIMITED
 COR. OF CLARENCE ST. & BRADDA AVE.
 SCALE - 1/4" = 1'-0"
 (1/8" = 1'-0" FOR DETAILS)

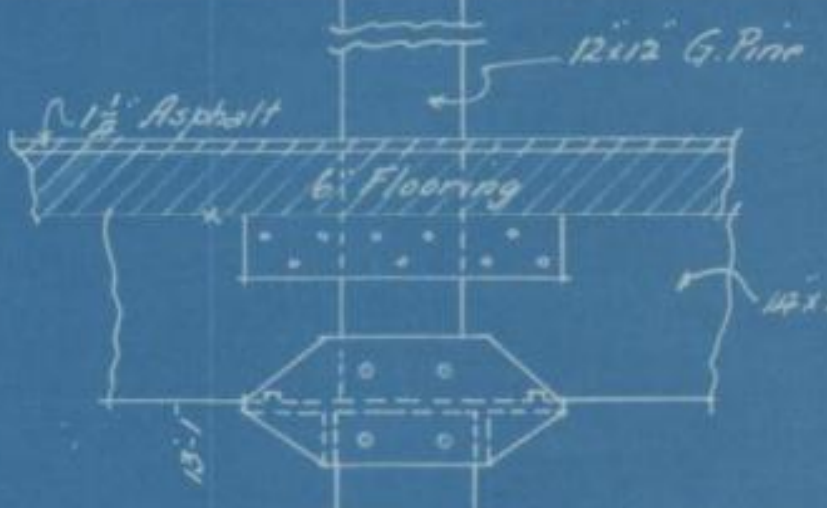
STRAUT & KILPATRICK ARCHT.



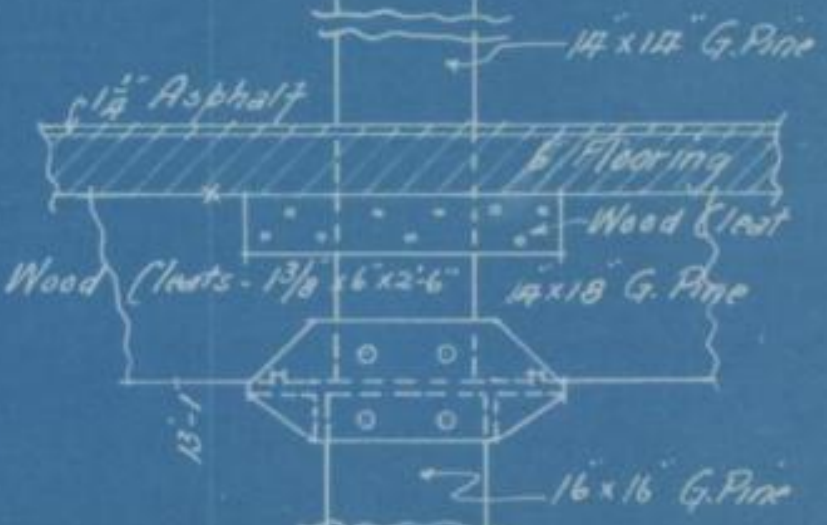
STRAUT & KILPATRICK ARCHT.
 1000 BRADDA AVE.
 PORTLAND, ME.



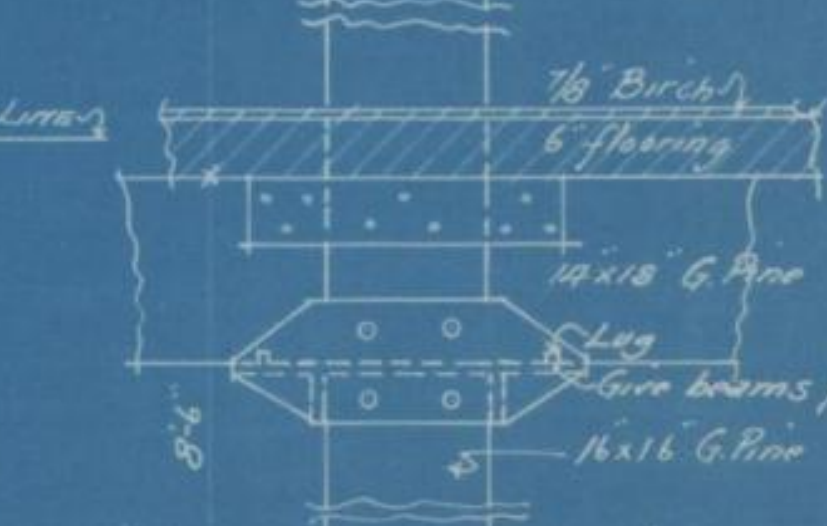
COURTESY OF THE PORTLAND PROPERTY



SECTION



DETAIL OF BOX ANCHORS



2 thicknesses of Asbestos paper

COURTESY OF THE PORTLAND PROPERTY GROUP

SPROATT & ROLPH,
ARCHITECTS,

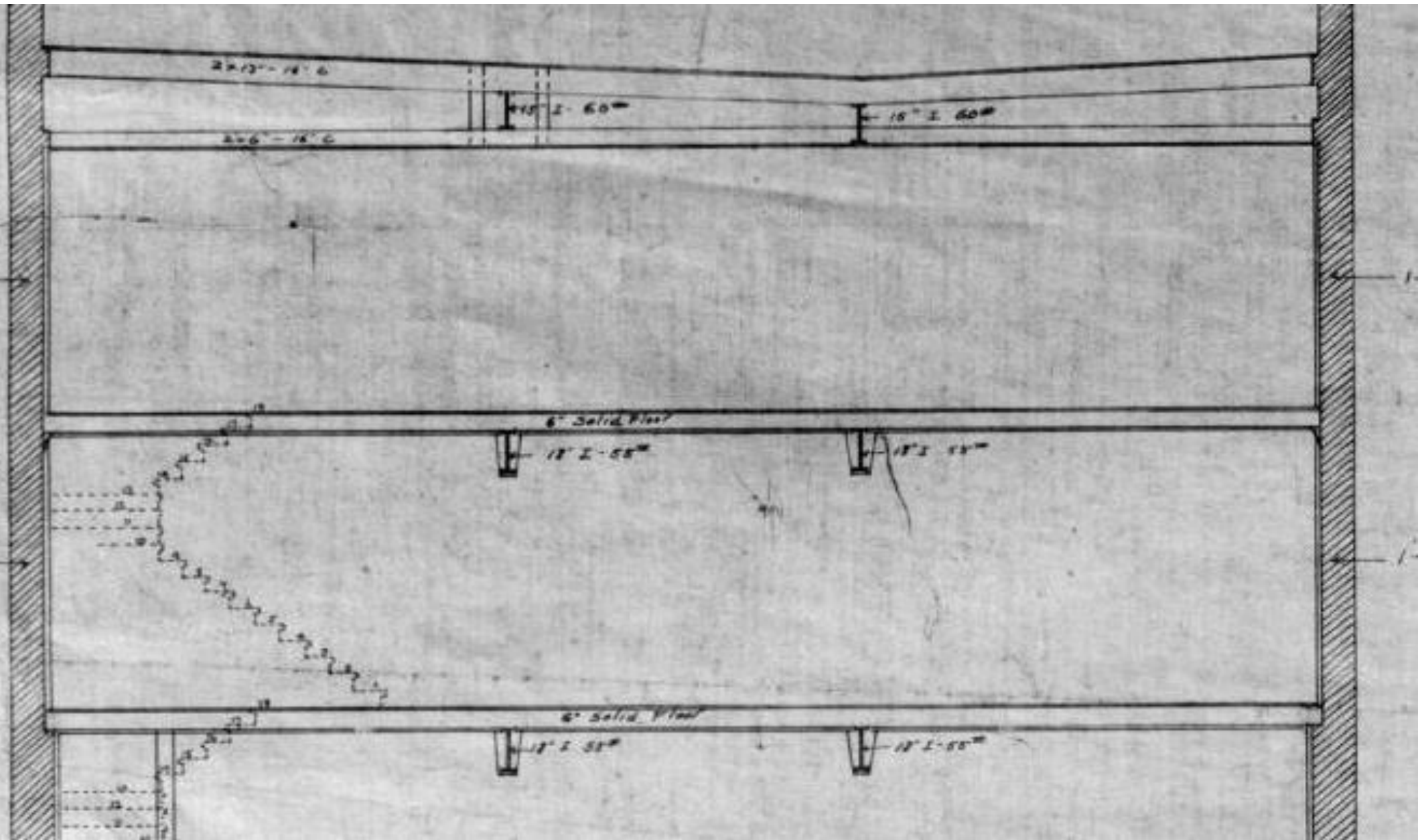
TORONTO.

CONTRACT No. 207.

SHEET No. 9.

DATE Aug. 11/1911.

Mass Timber on Steel



The Code Changed

Yeah! 6-storey?

Oh wait: 18m (59ft)

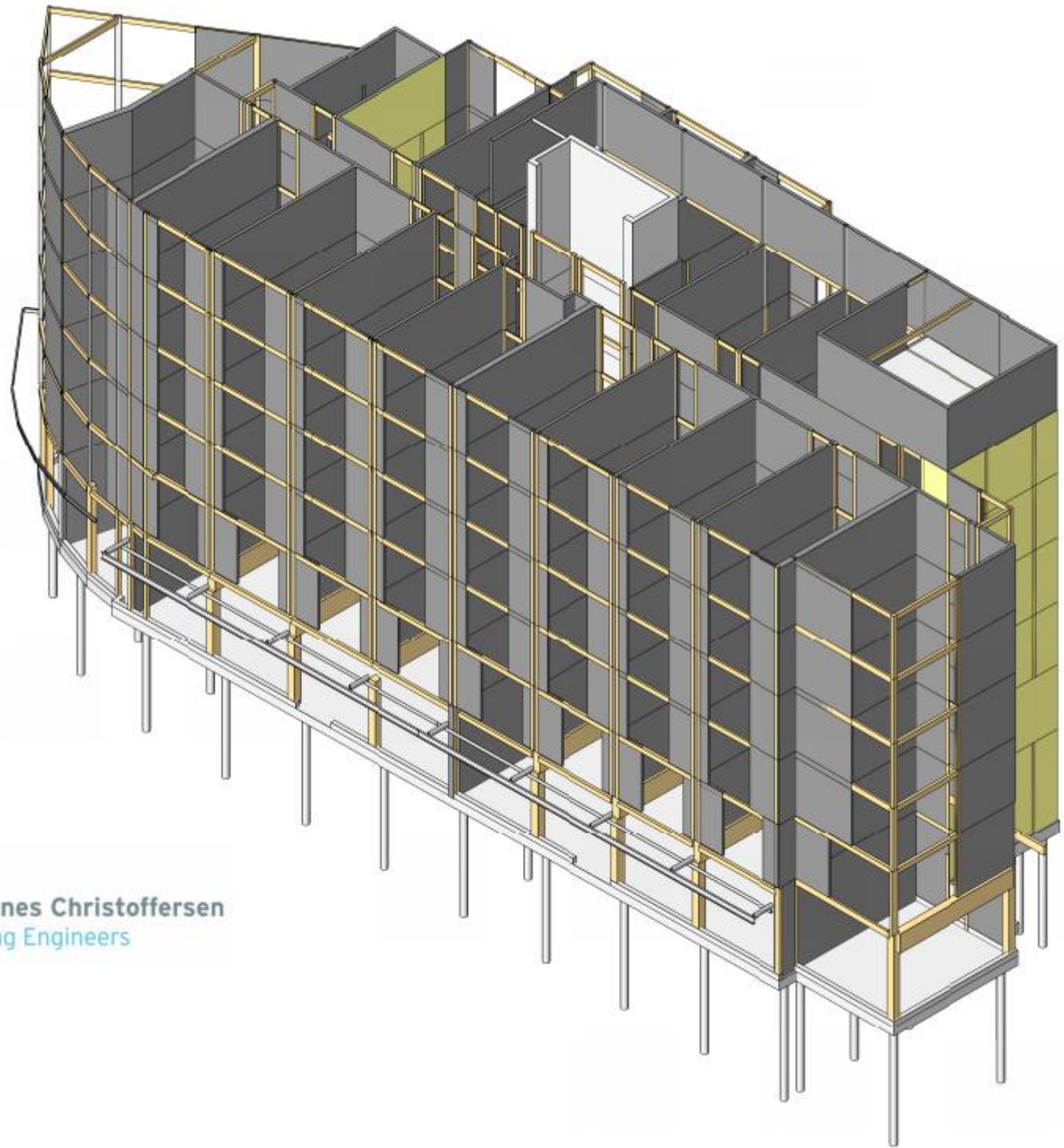
Made in Ontario ☹️

Non-combustible Egress Exits

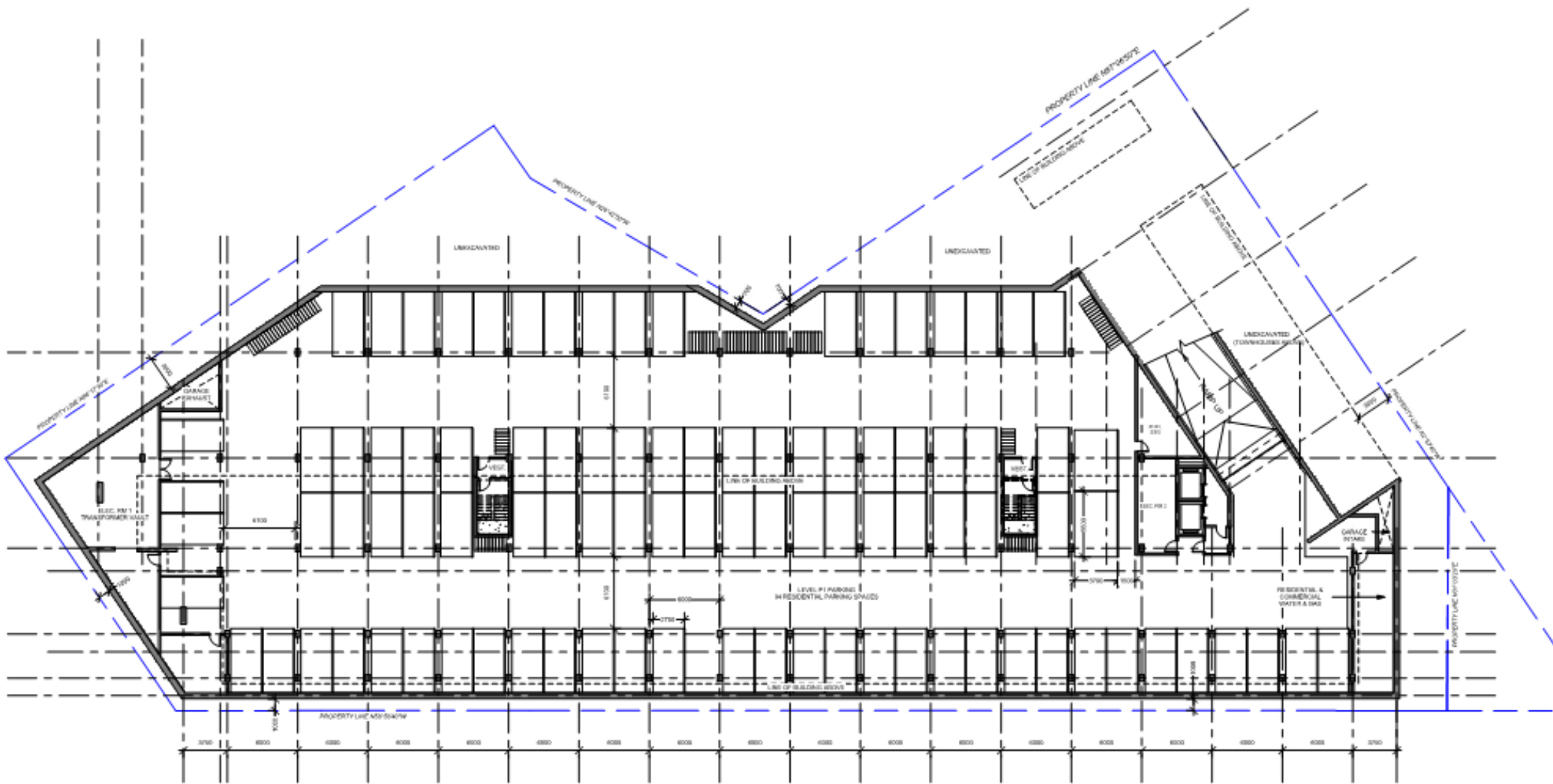
Mid-rise Construction in British Columbia

A CASE STUDY BASED ON THE REMY PROJECT IN RICHMOND, BC





Read Jones Christoffersen
Consulting Engineers



Lessons from 4-story Condos



PM12:51 NOV/25/2014



VANDYK 30
APART

VANDYK 30

VANDYK 30

VANDYK 30

MELO

MELO
LIFT.

M18044

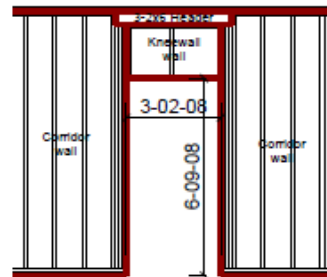
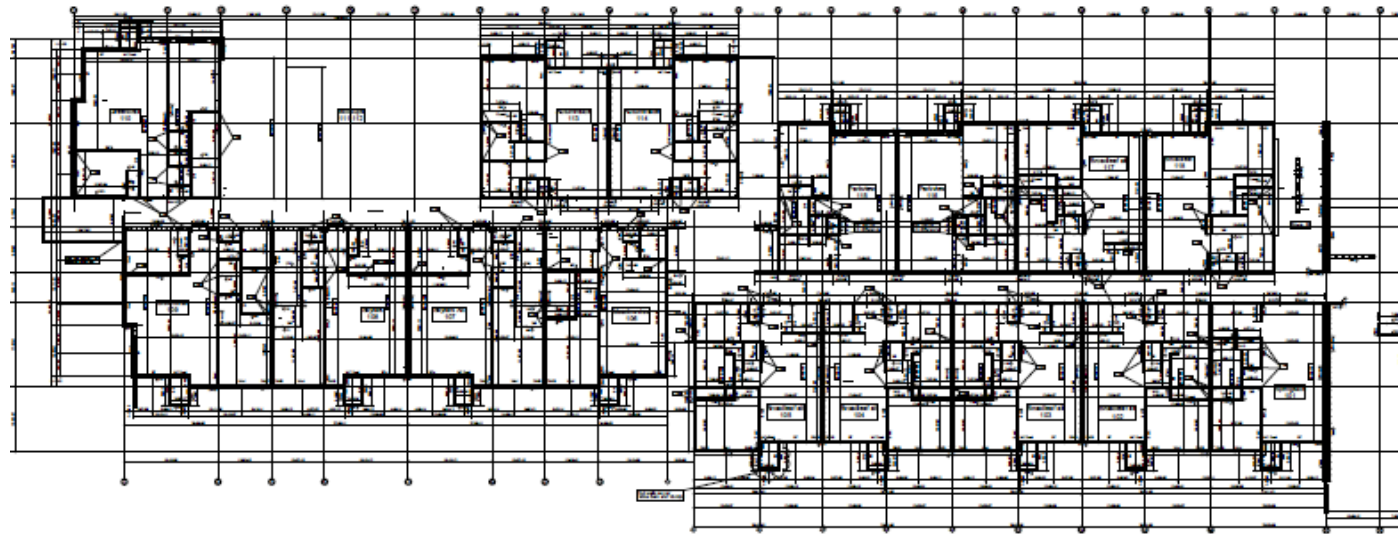
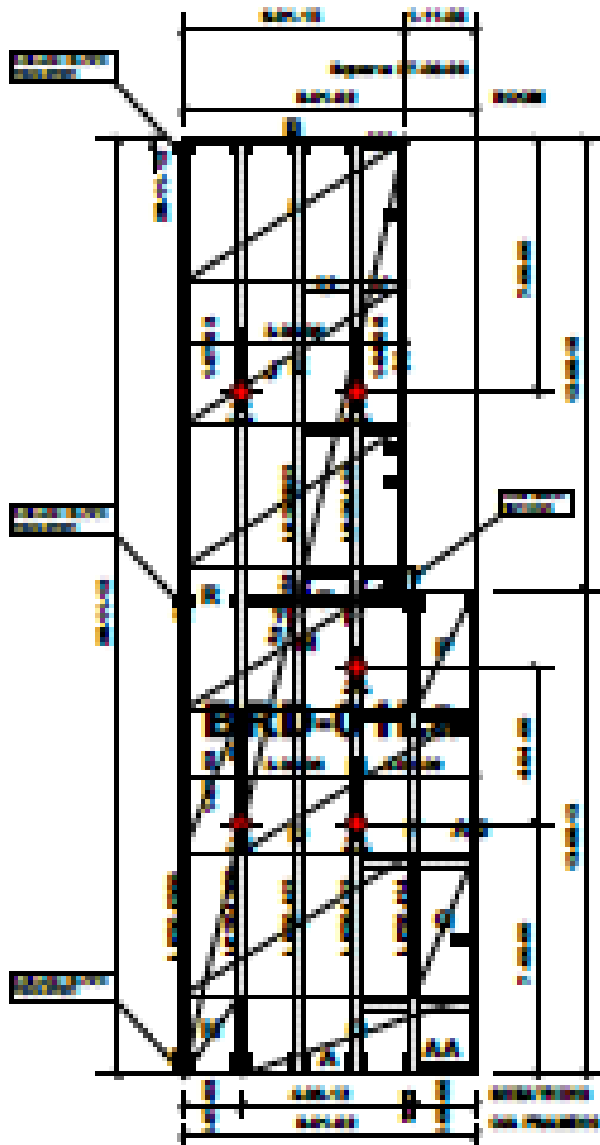
PM 12:34 NOV/25/



 **BROCKPORT**
HOME SYSTEMS LTD.



Panelize (Floors & Walls)

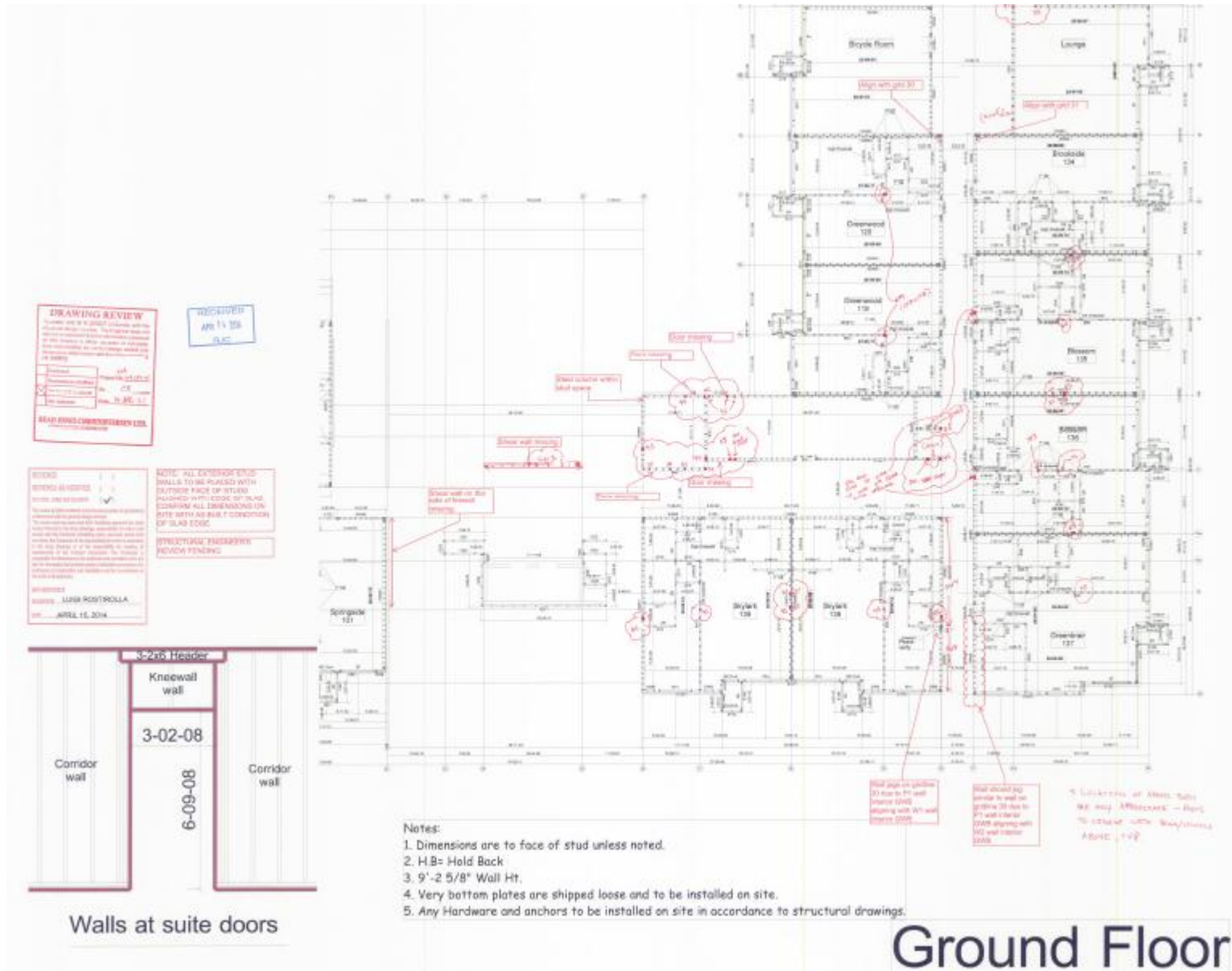


Walls at suite doors

- Notes:
1. Dimensions are to face of stud unless noted.
 2. H.B.= Hold Back
 3. 9'-2 5/8" Wall Ht.
 4. Very bottom plates are shipped loose and to be installed on site.
 5. Any Hardware and anchors to be installed on site in accordance to structural drawings.
 6. ● Indicates anchor locations.

Ground Floor
Phase 1

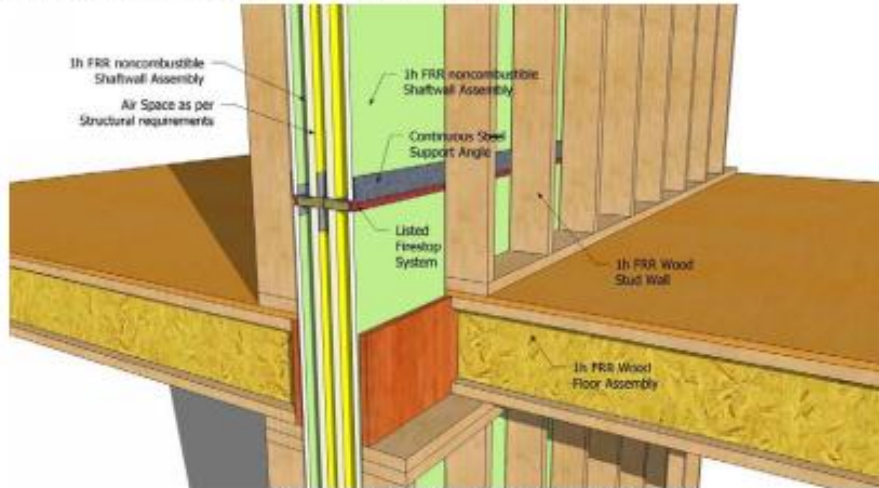
Coordination



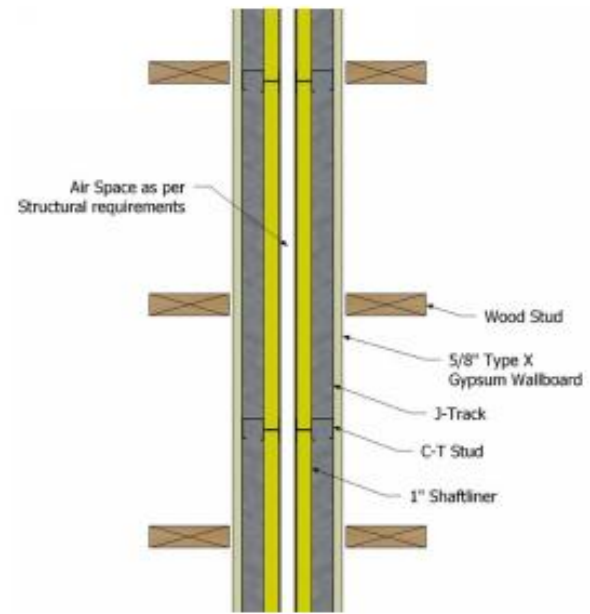
Fire-wall

To separate into two buildings.

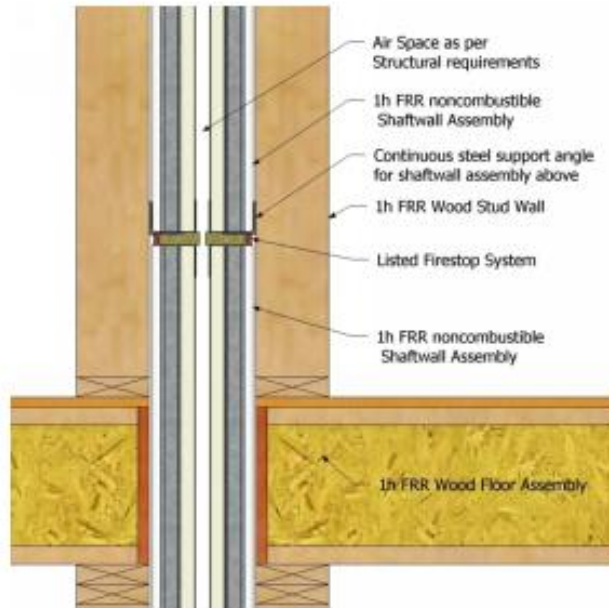
APPENDIX F: NON-COMBUSTIBLE WALL ASSEMBLIES



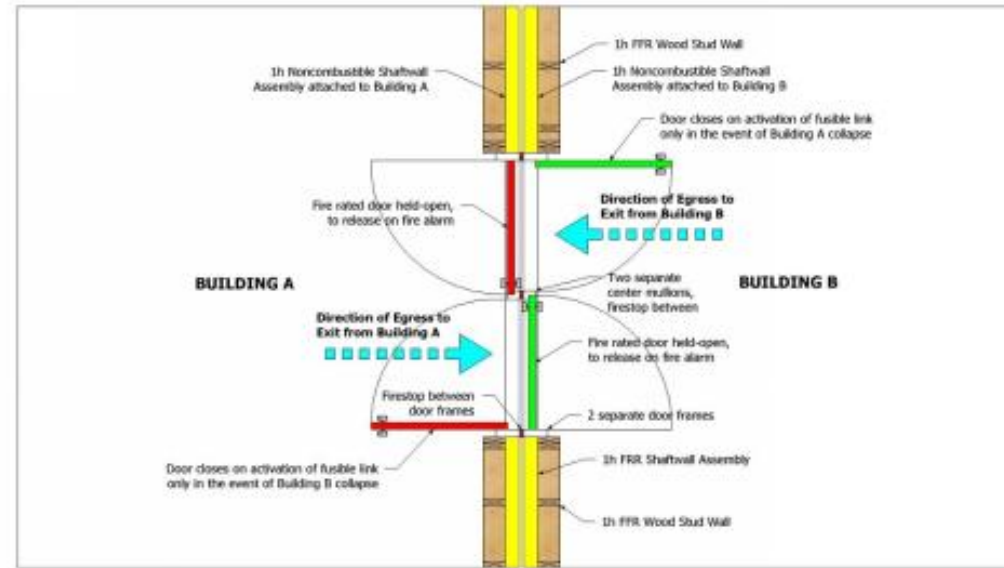
Firewall Using Two Shaftwall Assemblies
Perspective View



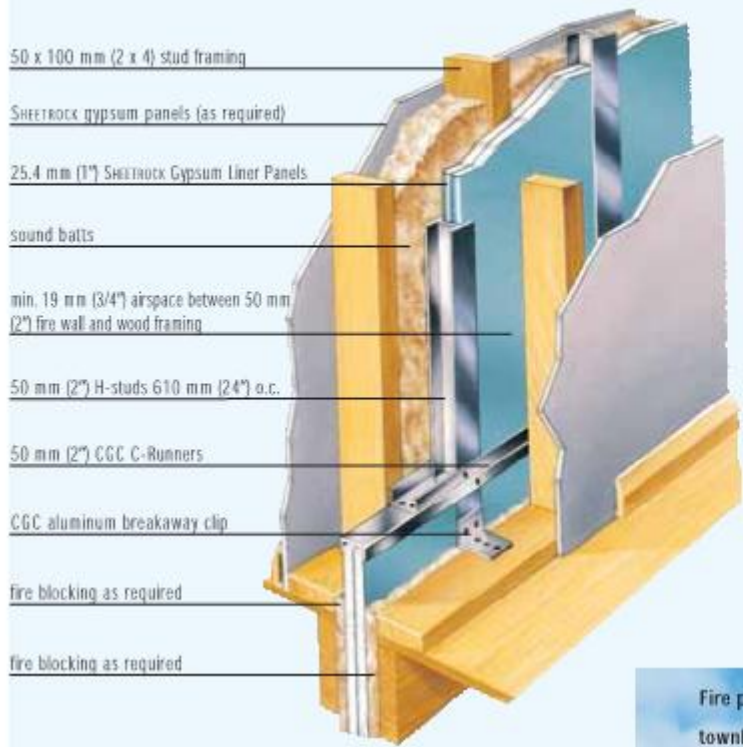
Firewall Using Two Shaftwall Assemblies
Plan View / Horizontal Section



Firewall Using Two Shaftwall Assemblies
Side View / Vertical Section



Door Arrangement at Firewall
Using Two Shaftwall Assemblies



50 x 100 mm (2 x 4) stud framing

Sheetrock gypsum panels (as required)

25.4 mm (1") Sheetrock Gypsum Liner Panels

sound batts

min. 19 mm (3/4") airspace between 50 mm (2") fire wall and wood framing

50 mm (2") H-studs 610 mm (24") o.c.

50 mm (2") CGC C-Runners

CGC aluminum breakaway clip

fire blocking as required

fire blocking as required

4 CGC Gypsum Fire Wall Systems

Fire protection for townhouses that share a common wall

CGC
A USG COMPANY



Gypsum Fire Wall

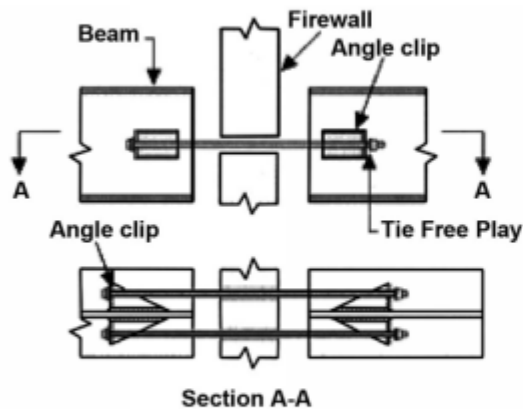
Systems

SA-925

ing frame and the unexposed frame. This can be achieved by using solid wall sections, as illustrated in Figures 5A.8 and 5A.9.

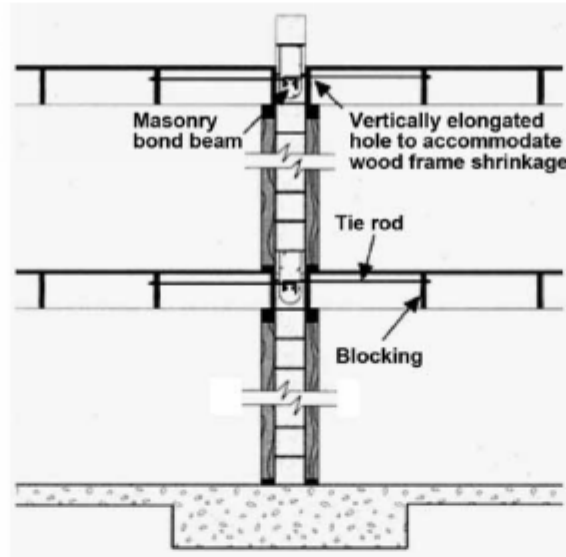
- Where tied firewalls encase steel columns, expansion of the steel framing members on the fire side of the wall will be resisted by the framing on the unexposed side of the wall. The connection of the columns to the wall should allow for movements which would occur in the protected frame when resisting the sagging force exerted by the fire-exposed frame. This can be achieved by using flexible masonry anchors or by using concrete block units that loosely key into the re-entrant space of the column.
- In all cases, the firewall itself must be designed to withstand the lateral loads specified in NBCC-10 Article 4.1.5.17.

Figure 5A.20: Through-Wall Tie, Primary Steel Perpendicular to Tied Firewall



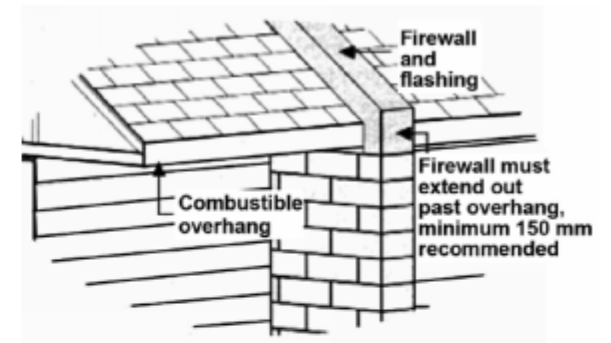
act as a weak link in accordance with Paragraph 15 of Commentary "C" of "User's Guide—NBC 2010, Structural Commentaries (Part 4 of Division B)". The firewall itself must be reinforced and detailed in accordance with Paragraphs 8, 9, 14 and 15 of Commentary "C". This form of construction is typically used in wood frame multi-unit residential buildings where firewalls are used to separate dwelling units or building sections.

Figure 5A.21: Weak Link Connection Firewall



An alternative form of the weak link connection can be used where wood floor joists run perpendicular to, and are supported on, the firewall. The ends of the joist should be fire cut as shown in Figure 5A.22. This will enable the floor framing exposed to the fire to disen-

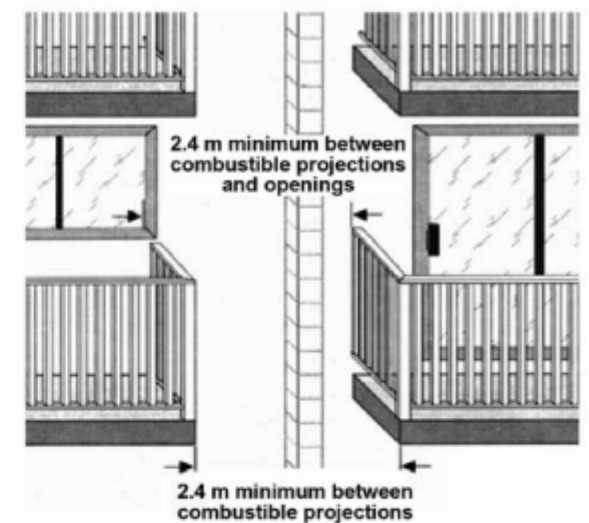
Figure 5A.27d: Firewall Extension Past Eave



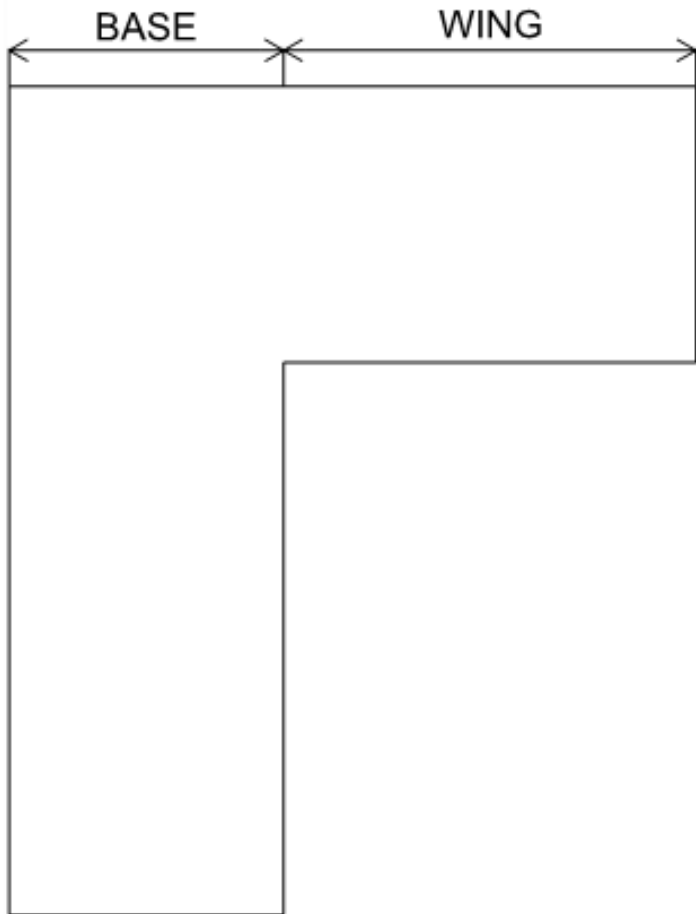
5A.5.7 Combustible Projections

Combustible projections such as balconies, platforms, stairs and eaves that are located near a firewall can also be ignited by flames or heat that pass around the end of a firewall. Therefore, combustible projections are not permitted within 2.4 m of similar combustible projections, or window or door openings, placed on the opposite side of the firewall. This distance should provide adequate separation to prevent ignition of combustibles [3.1.10.7.(2), NBCC-10] (Figure 5A.28).

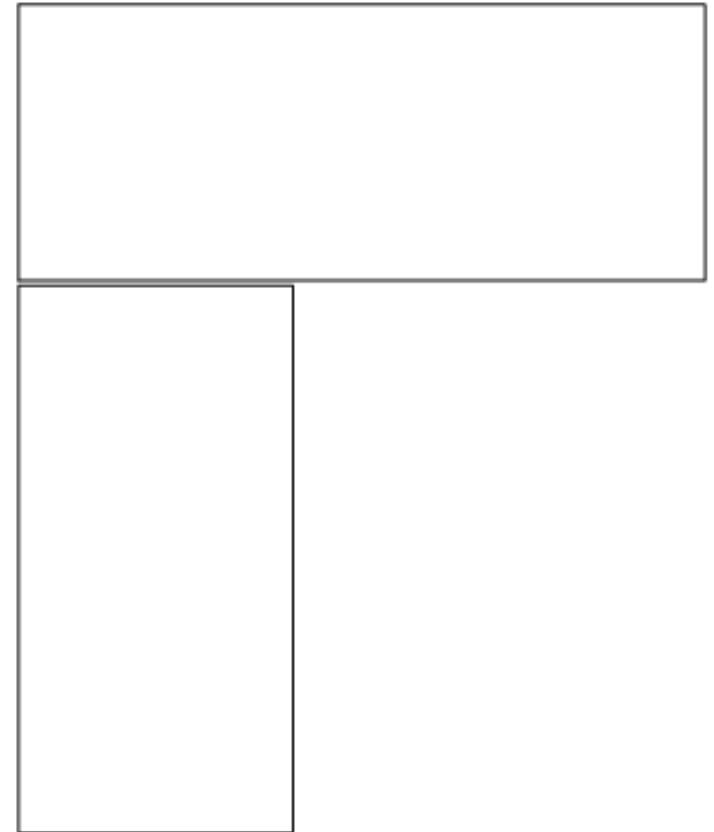
Figure 5A.28: Combustible Projections



Firewalls



IF WING > BASE →



Plan layout where the wing has a length greater than the base width

Party-wall (Shear walls)

Between Suites
(Fire & Acoustics)

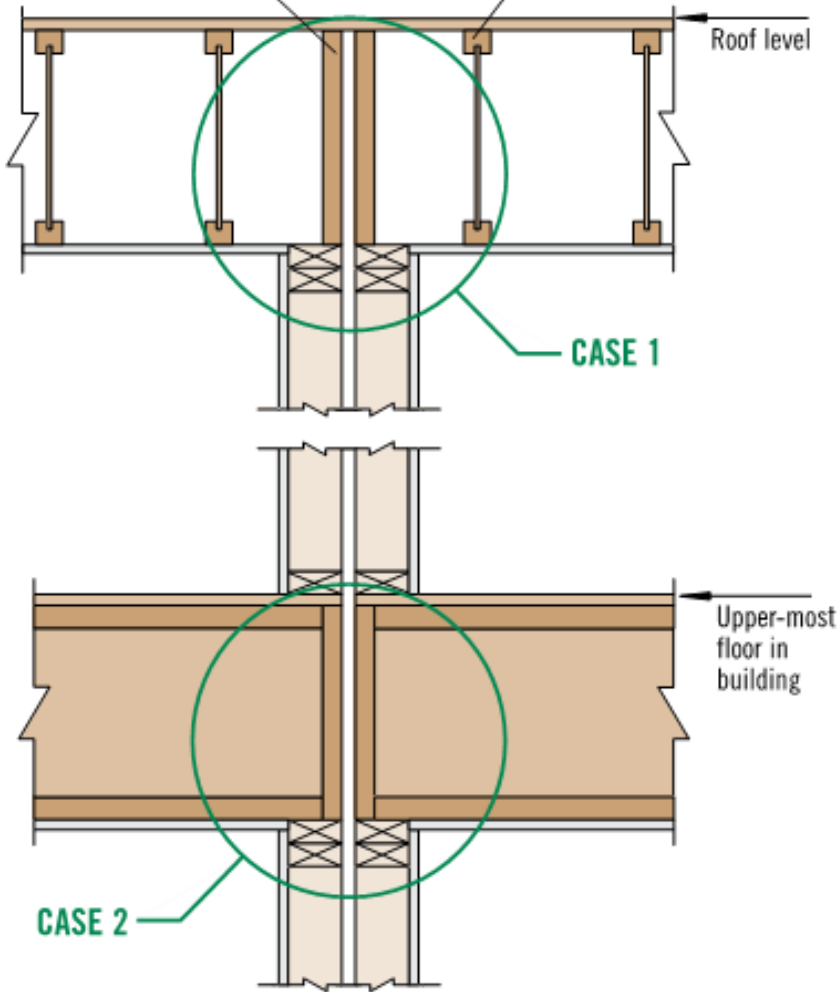
Must be aligned vertically, in plan.

Must be continuous from Fdn to Roof.

TJI® Roof Joists Parallel to Wall

If using alternative roof framing materials, such as plated trusses, consult architect and/or roof truss manufacturer for required draft-stopping and membrane protection

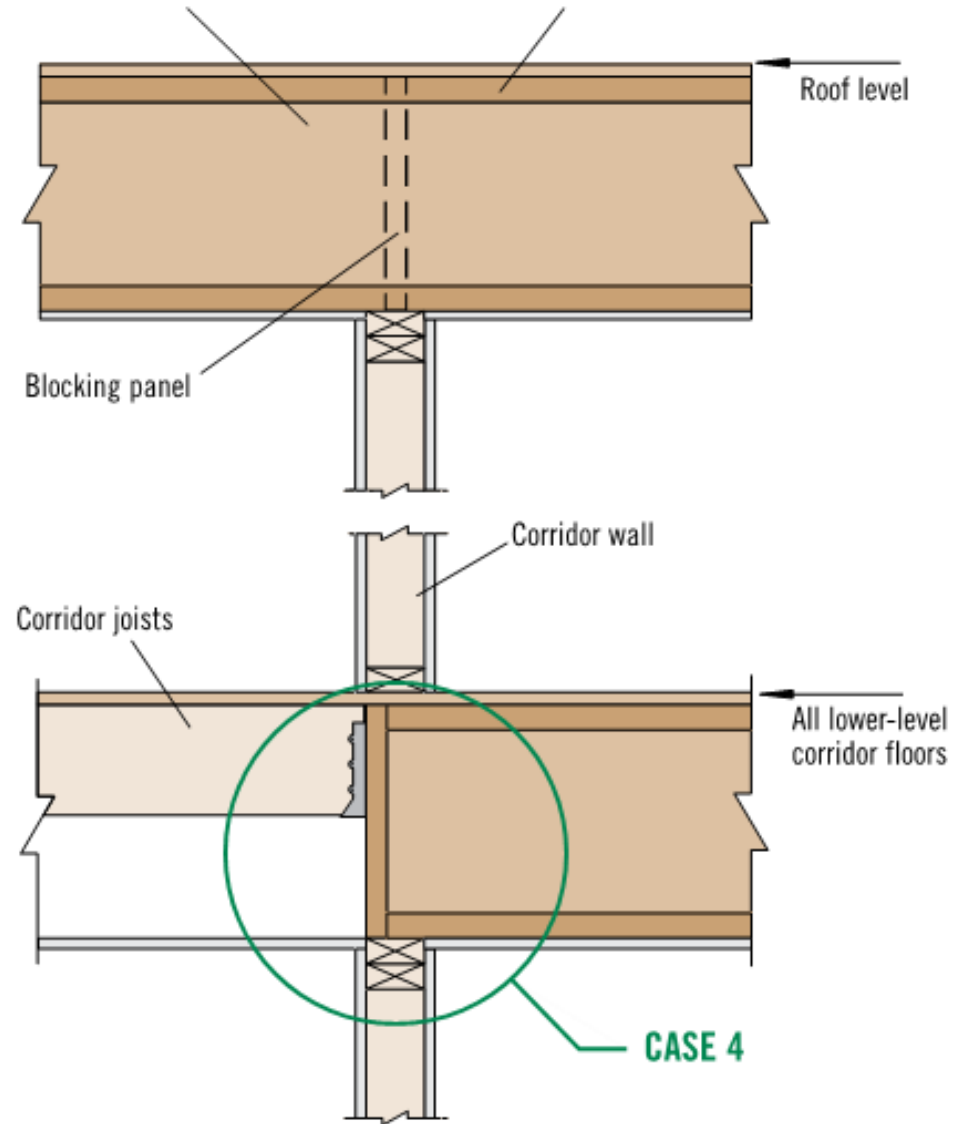
Only rim board bears on wall

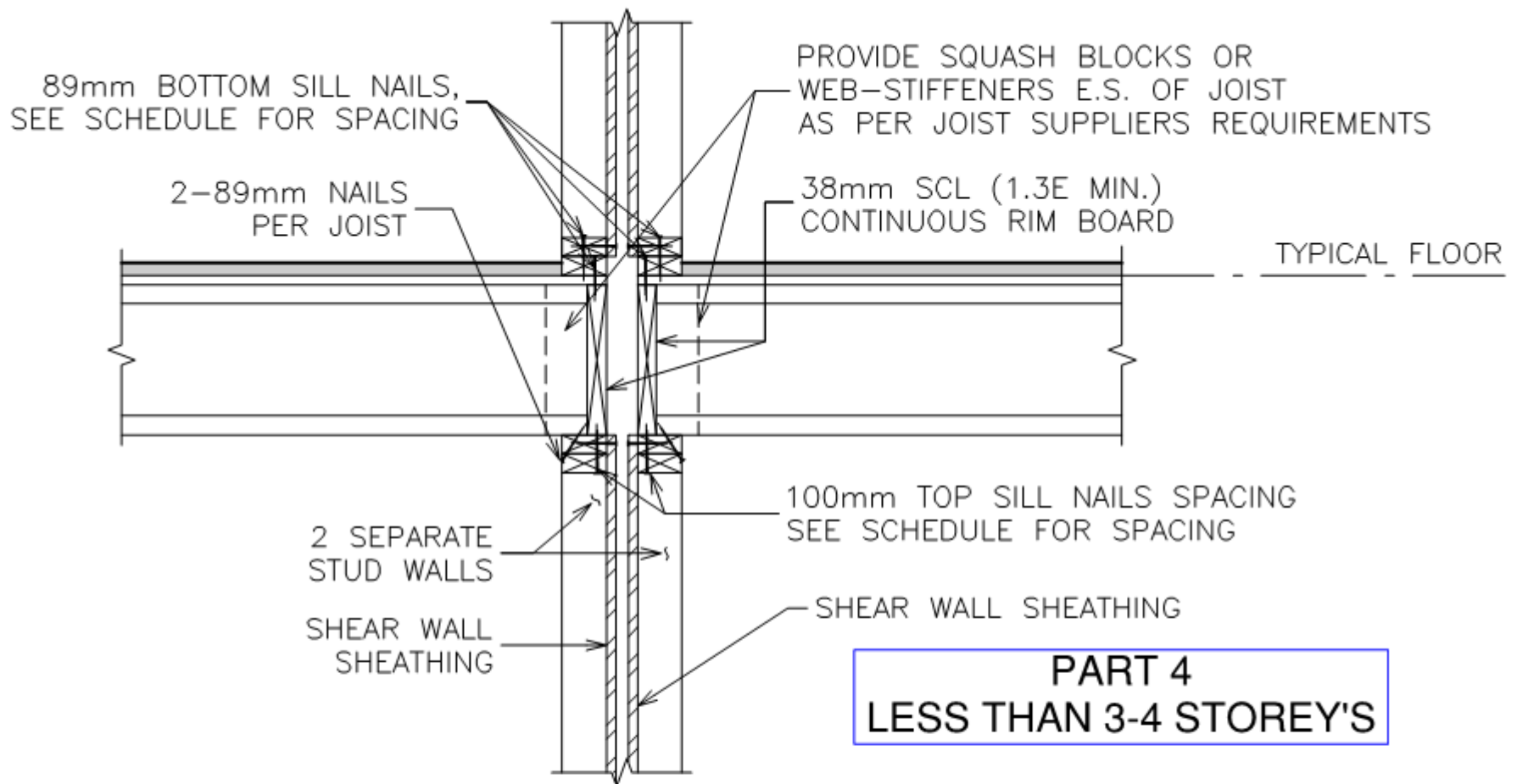


TJI® Joists at Corridor Walls

If using alternative roof framing materials, such as plated trusses, consult architect and/or roof truss manufacturer for required draft-stopping and membrane protection

TJI® joists are bearing on wall





11
S401

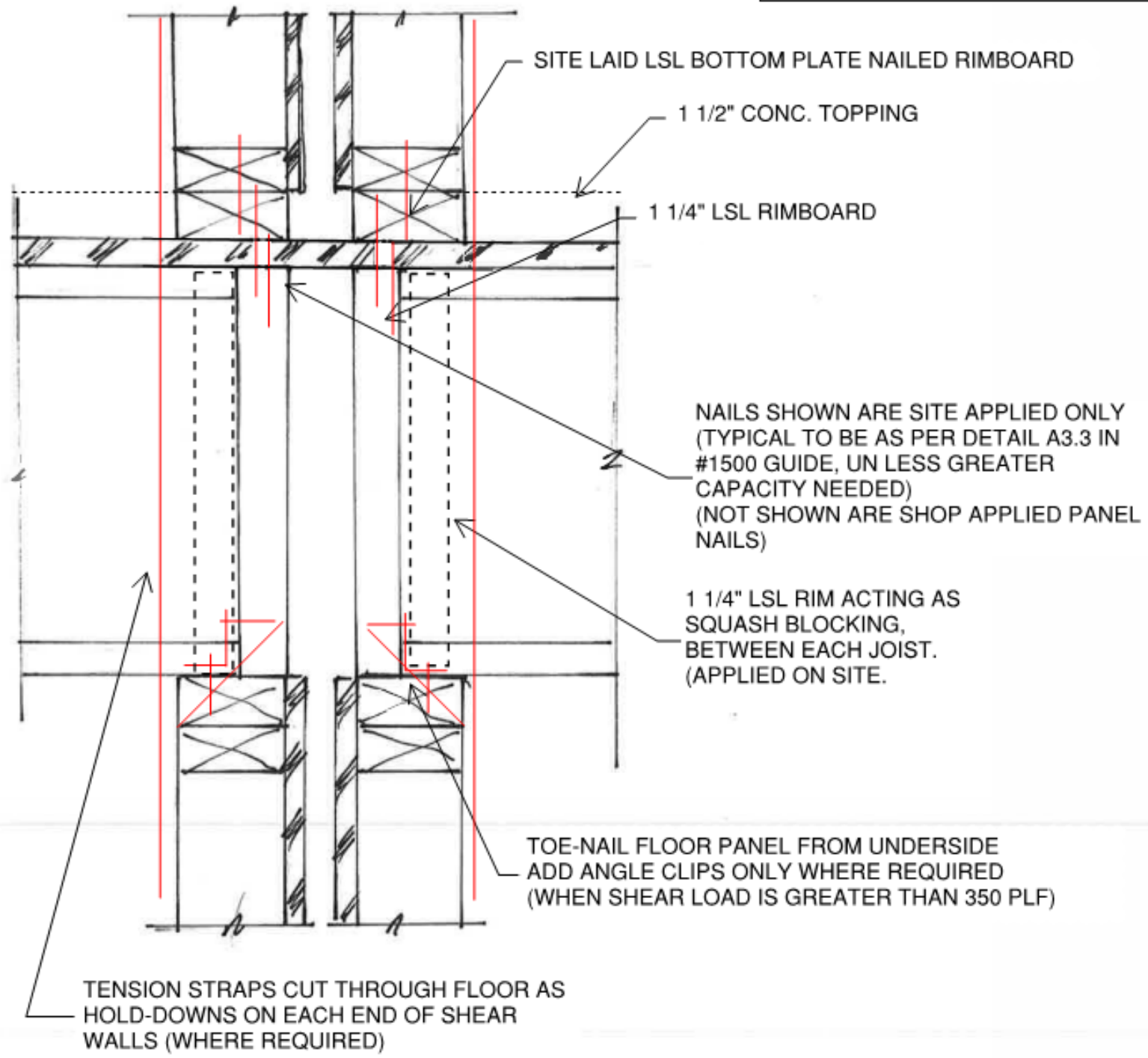
**TYPICAL AT
PARTY WALL**

1:20



Read Jones Christoffersen
Consulting Engineers

TYPICAL PARTY WALL DETAIL



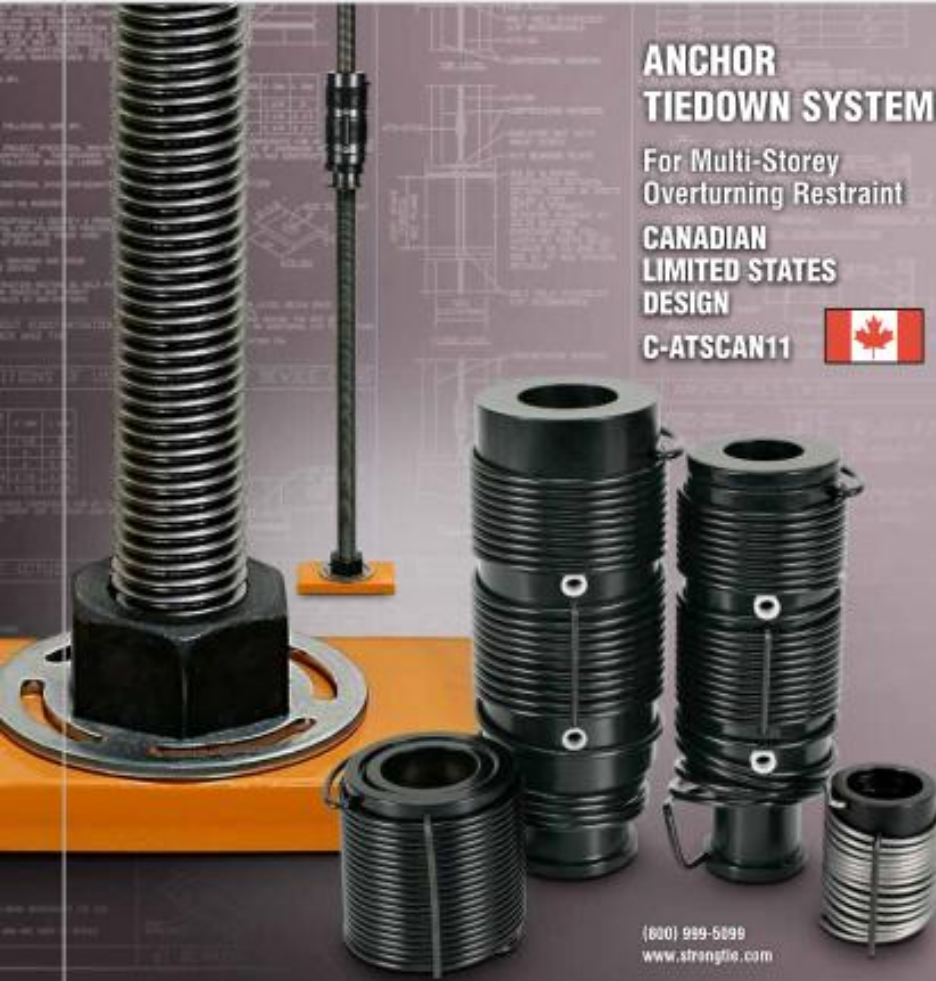


ANCHOR TIEDOWN SYSTEM

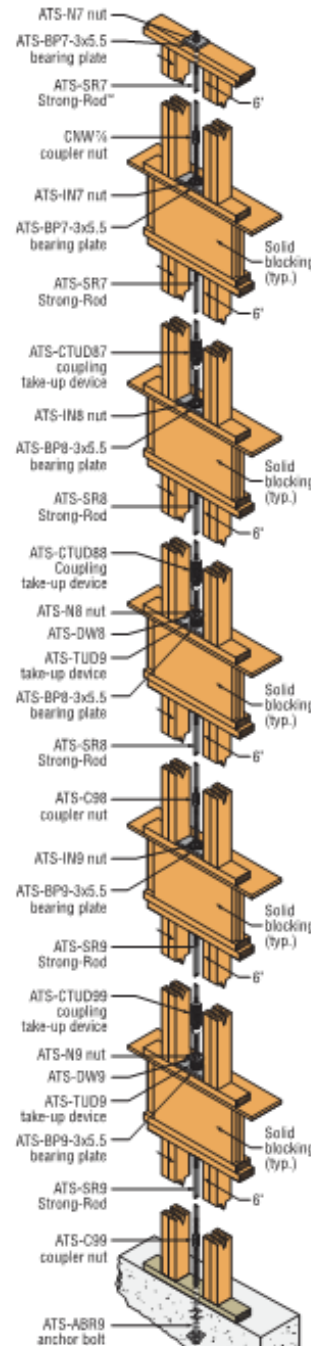
For Multi-Storey
Overturning Restraint

CANADIAN
LIMITED STATES
DESIGN

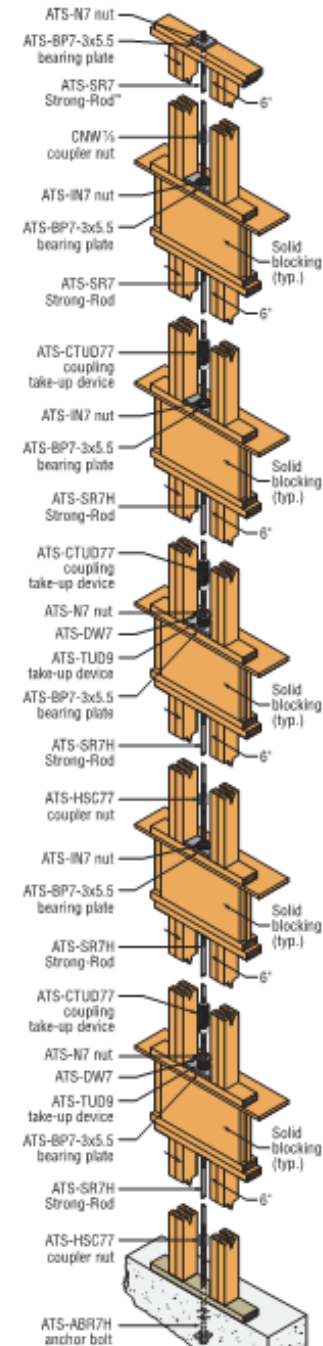
C-ATSCAN11



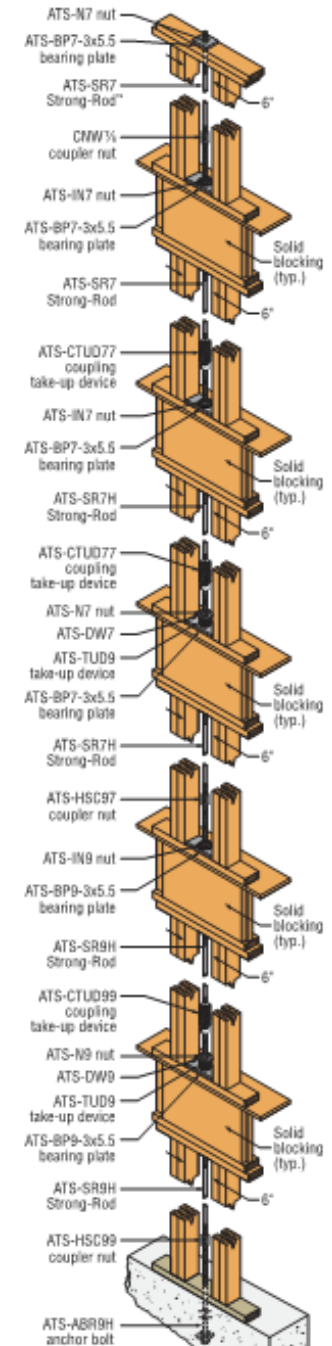
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CTDS6-1 1/8



CTDS6-7/8H



CTDS6-1 1/8H

Urban Office Space

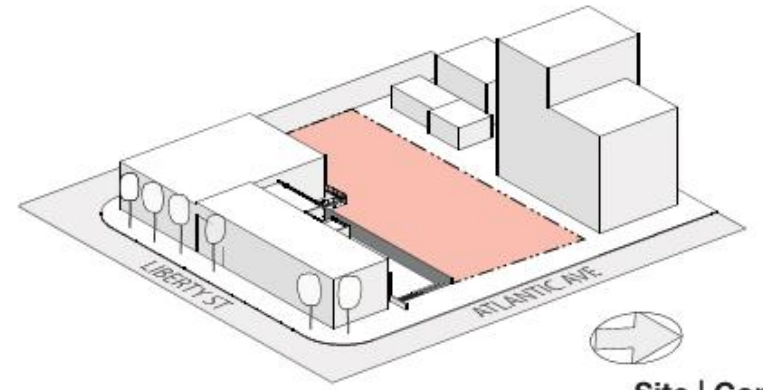
Mass/Heavy Timber

on

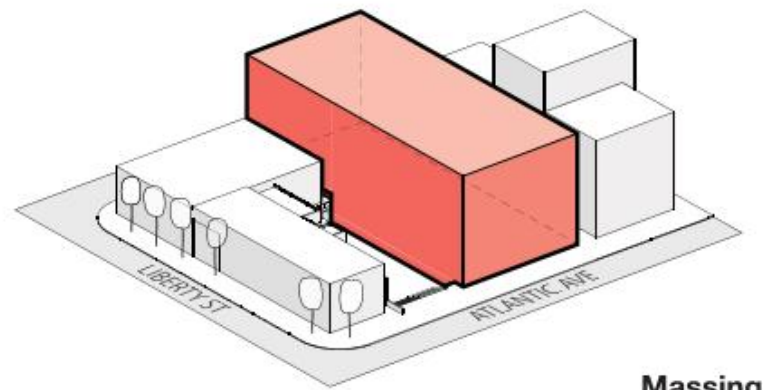
Concrete Podium & Parking



Site



Site | Context



Massing

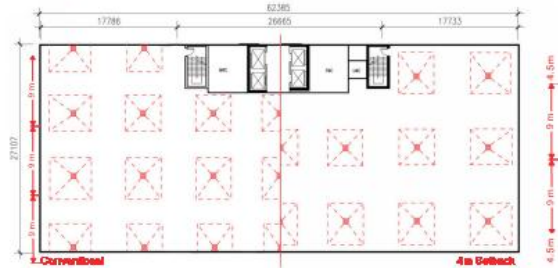
Concept / Preliminary Design

5 or 8 stories?

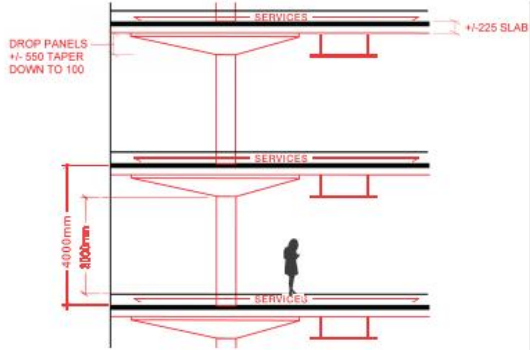
Conceptual Structural Options



1. Reinf. Conc. Cant. Slab



1a. Reinforced Concrete Cantilevered Slab



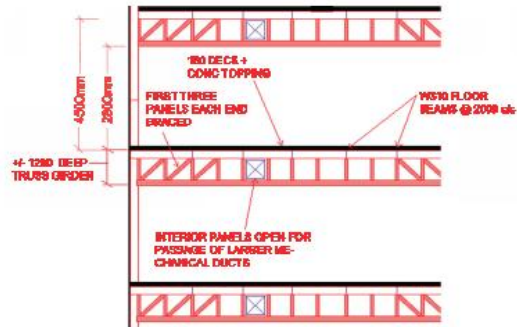
1b. Reinforced Concrete Cantilevered Slab



2. Steel Truss Girders



2a. Steel Truss Girders



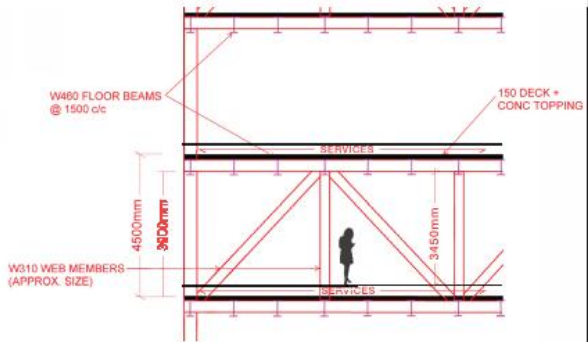
2b. Steel Truss Girders



3. Full Storey Truss



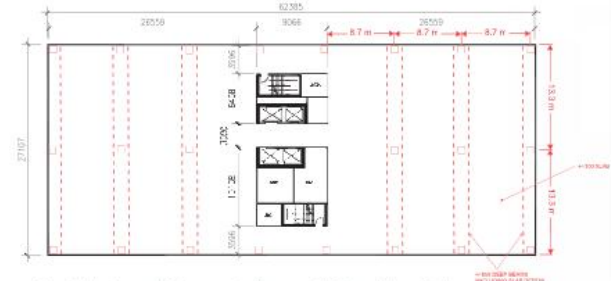
3a. Full Storey Truss



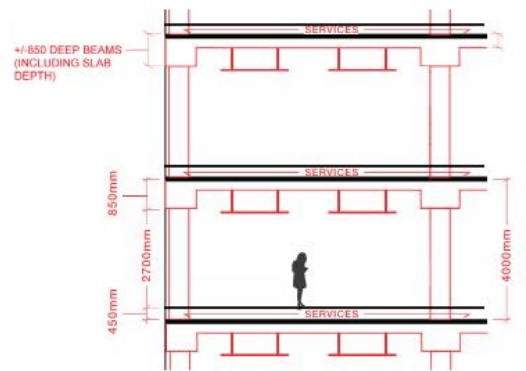
3b. Full Storey Truss



4. Reinforced Concrete Beams & One-Way Slabs



4a. Reinforced Concrete Beams & One-Way Slabs

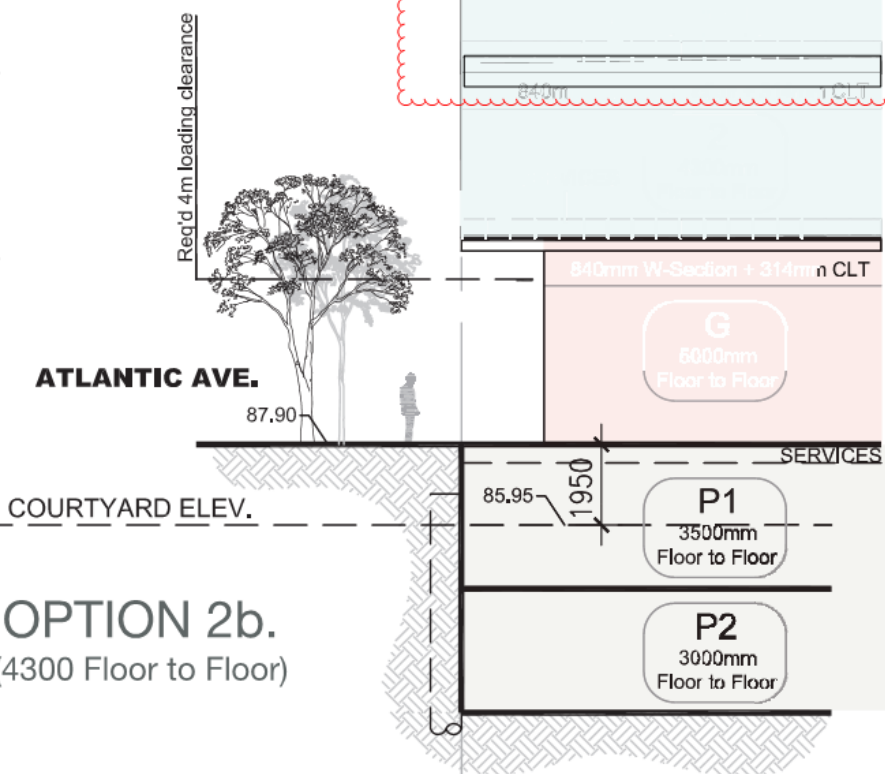


4b. Reinforced Concrete Beams & One-Way Slabs

Content Provided By RJC Consulting Engineers

ISSUED FOR
STRUCTURAL WOOD
BUDGET PRICING
& DISCUSSION

MAX. WOOD BLDG. FLR. HEIGHT
MAY. (18.0m / 59 ft)



ROOF (GREEN ROOF)
(PRICE SAME AS TYPICAL FLOOR)

3 TYPICAL OFFICE FLOORS OF CLT / GLULAM
(1 HR FRR - ALL CONNECTORS STEEL HIDDEN)

4 LEVELS OF
GLULAM COLUMNS



4300
TYP. F.F. TO F.F.

2ND FLOOR
CONCRETE PODIUM SLAB

UNDER SIDE OF EXPOSED TO VIEW
TOP SIDE (CONCRETE TOPPING)

CAST IN PLACE CONCRETE

PURLINS:
365x874 D. FIR 24f-E GLULAM

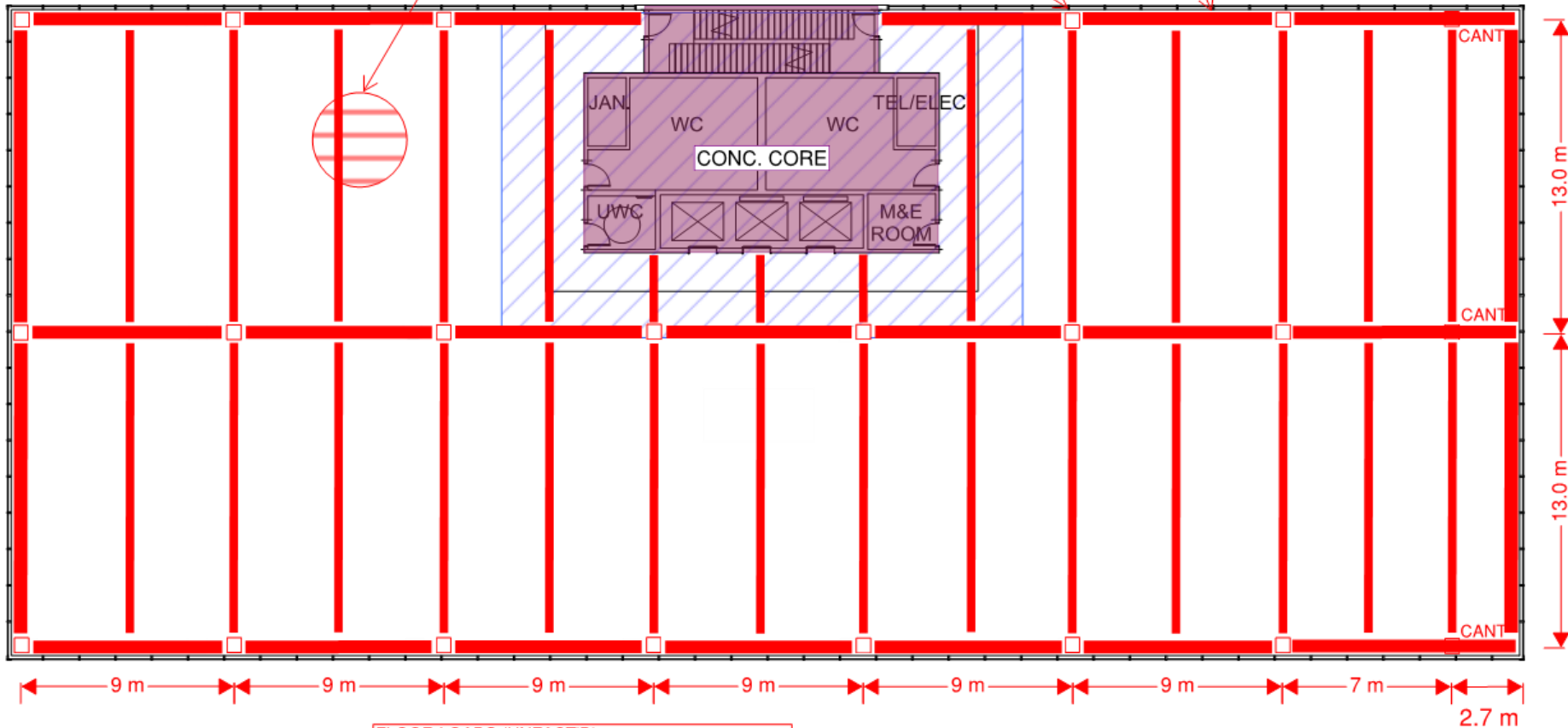
CENTRAL GIRDERS:
530x874 D. FIR GLULAM
(DOUBLE 265x874)

PERIMETER GIRDERS:
530x874 D. FIR GLULAM
(DOUBLE 265x874)

+/-162 DP. CLT PANELS
w/ 38 mm CEMENTITIOUS TOPPING

TYP. COLUMNS:
530x532 D. FIR GLULAM

OPTION 1



FLOOR LOADS (UNFACT'D):

SDL = 3.0 kPa (INCLUDES 38 mm CONC. TOPPING)
 LL = 3.85 kPa TYP. AT OFFICE,
 = 4.8 kPa AT CORRIDOR, NOTED THUS:

CLADDING LOADS (UNFACT'D):
 (PRELIMINARY, TO BE VERIFIED)
 WEST, NORTH & EAST ELEVATIONS: 17.5 kN/m
 SOUTH ELEVATION: 5.5 kN/m



Project		
Date	23 FEB, 2015	Page PSK-3
Designer	CB/MR	

MAIN BEAMS:
630x1216 D. FIR GLULAM
(DOUBLE 315x1216)

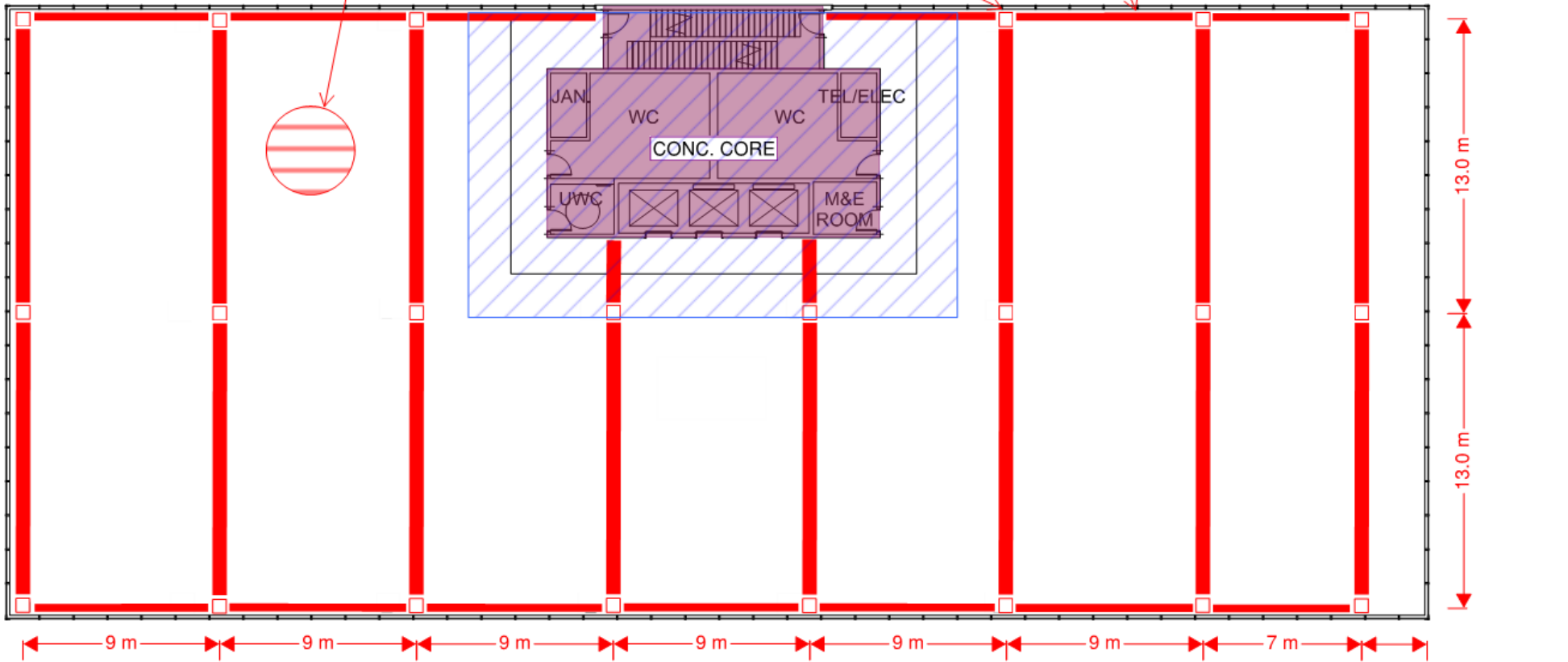
CLADDING SUPPORT BEAMS:
265x418 D. FIR GLULAM


OPTION 2



+/-300 DP. CLT PANELS
w/ MIN 100 mm COMPOSITE
CONCRETE TOPPING

TYP. COLUMNS:
630x608 D. FIR GLULAM



FLOOR LOADS (UNFACT'D):
 SDL = 2.0 kPa (NOT INCLUDING CONC. TOPPING)
 LL = 3.85 kPa TYP. AT OFFICE,
 = 4.8 kPa AT CORRIDOR, NOTED THUS: 

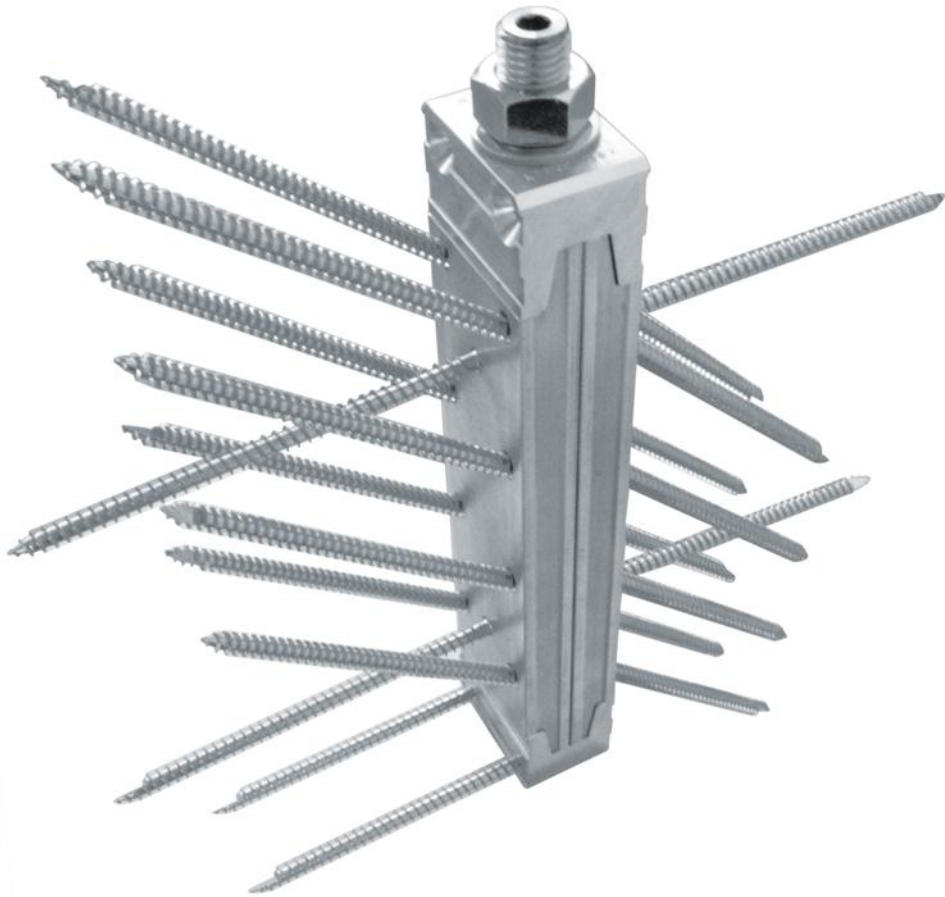
CLADDING LOADS (UNFACT'D):
 (PRELIMINARY, TO BE VERIFIED)
 WEST, NORTH & EAST ELEVATIONS: 17.5 kN/m
 SOUTH ELEVATION: 5.5 kN/m



Project		
Date	23 FEB, 2015	Page PSK-4
Designer	CB/MR	



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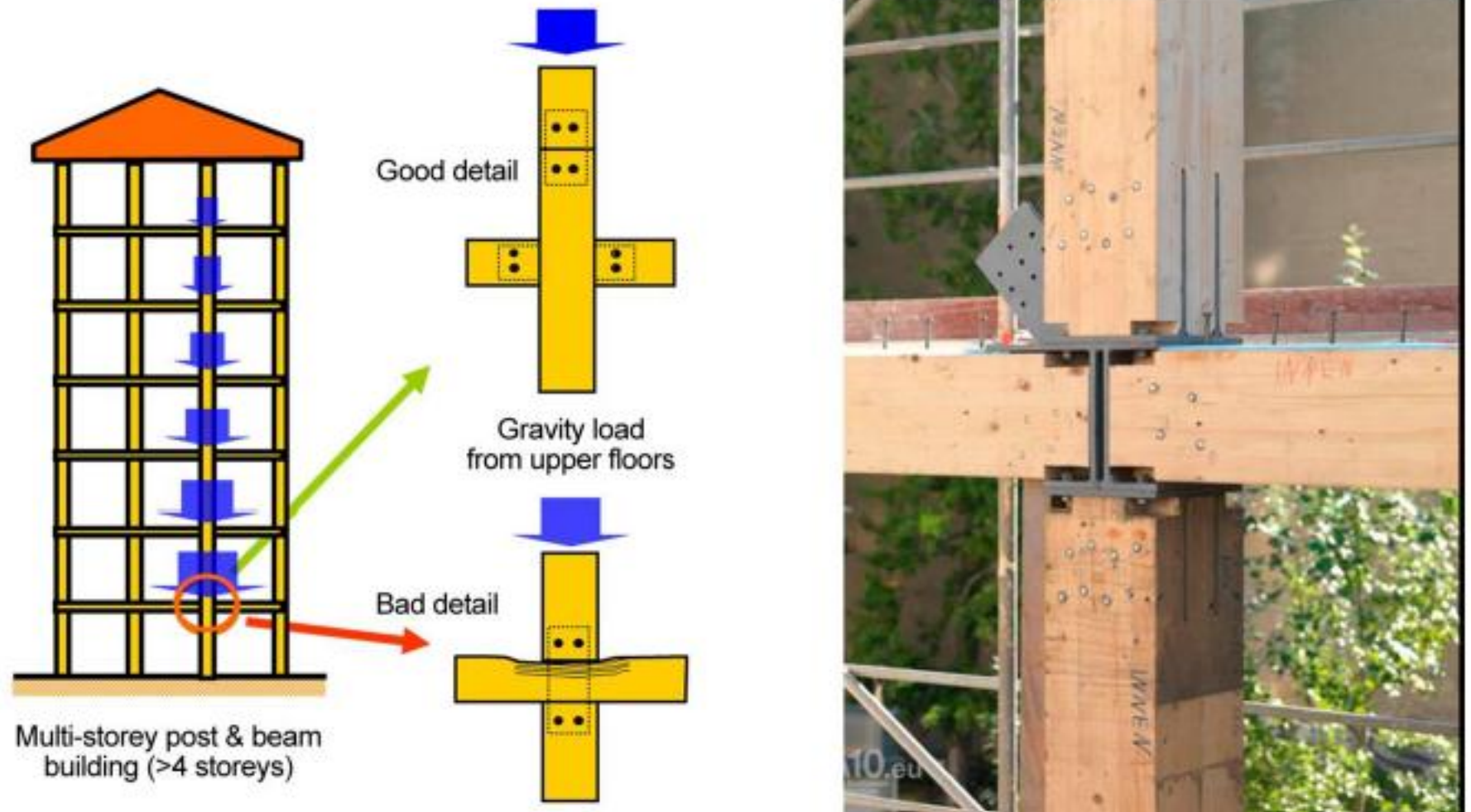


Figure 2 Post to beam connection details for avoiding excessive compression perpendicular to grain due to gravity loads

Bullitt Center, Seattle



Largest Gulam

60 ft long x 7ft tall





Recommendations

- Concept Design Phase (SPA)
- Semi-Prefabrication (DD)
- Economy comes from Simplicity
- Attention to the Details
- Coordination between all trades
- Commercial Project



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Consulting Engineers

Let's Discuss!

Thank You.



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