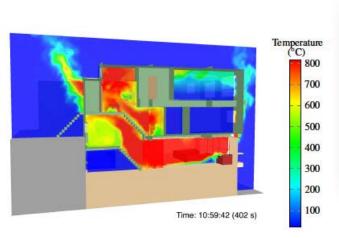
## Wood Construction and Part 3 of the NBC

#### PAST SUCCESSES AND FUTURE OPPORTUNITIES







Charlottetown, PE and Fredericton, NB September 14<sup>th</sup> - 15<sup>th</sup>, 2016











rtlett neering

## HOUSEKEEPING



Cell Phones
Fire / Emergencies
Questions











## **AGENDA**



- Wood Construction and the NBC/NFC
- Past Successes (Alternative Solutions)
- Future Opportunities
- Available Tools and Calculations
- Questions





## WHO WE ARE



## ESTABLISHEED 1987 Fredericton & Halifax

- Fire Protection Engineering
- Building & Fire Code Consulting
  - Fire Safety Planning
  - Forensic Investigations





## WHERE WE WORK

#### **Provinces include:**

Newfoundland

Nova Scotia

Prince Edward Island

**New Brunswick** 

Quebec

Ontario

Manitoba

Saskatchewan

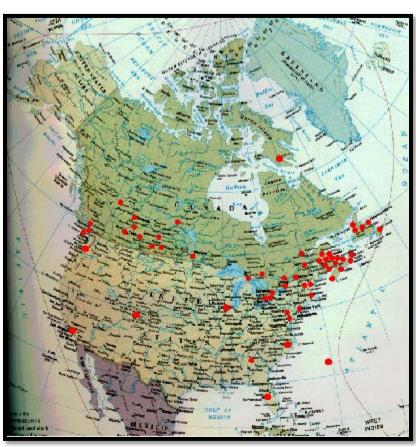
Alberta

**British Columbia** 

#### **Territories include:**

Nunavut

**Northwest Territories** 



#### International:

**Barbados United Kingdom** 

Bermuda Germany



#### States include:

Maine

Pennsylvania

Maryland

New York

Massachusetts

New Hampshire

Georgia

Florida

Washington DC

Illinois

California

North Carolina

Seattle





## **BRIEF BIOGRAPHY**

## Ben Coles, M.Sc.Eng., MBA, P.Eng., PE Project Coordinator

- Fire Protection Engineering
- Building & Fire Code Consulting
- 13 years experience

B.Sc. Mechanical Engineering (UNB 2003)

M.Sc. Fire Protection Engineering (WPI 2009)

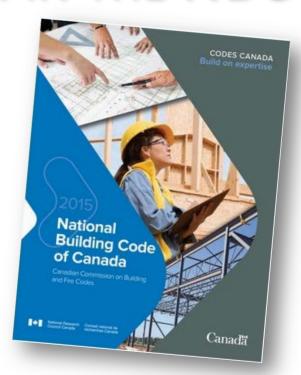
MBA in Engineering Management (UNB 2011)





# NBC CHANGES Six-storey wood structures in 2015 NBC

- Safety
- Limitations are being lifted



Sprinkler/FAS buildings just as safe with wood construction Non-combustible = better FLS is being challenged





# NBC CHANGES Six-storey wood structures in 2015 NBC

- Group C and D occupancies
- Changes address:
  - Sprinkler requirements
  - Area limitations

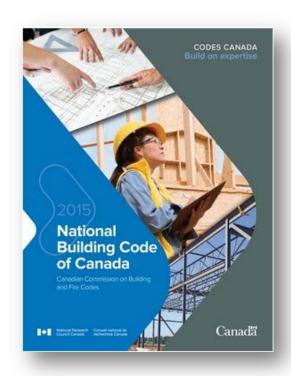






## APPENDIX D Fire-Performance Ratings

- Wood/Steel Framed Walls, Floors/Roofs
  - New materials and assemblies
  - Component Additive Method (CAM)
  - Time assignments for materials
- Fire test reports published by the NRC
  - Available online







## APPENDIX D Fire-Performance Ratings

Expansion and addition of tables in Appendix D-2.3.





#### Time Assignee for Contribution of Wood-Framed or Cold-Formed-Steel-Framed Walls

Description c rame	Time, min	
Description C Tame	Loadbearing Walls	Non-Loadbearing Walls
Wood studs spaced ≤ 400 mm o.c.	20	
Wood studs spaced ≤ 600 mm o.c.	15	
Cold-formed-steel studs spaced ≤ 400 — 1 o.c.	10	
Cold-formed-steel studs spaced ≤ 600 → o.c.	10	Manual -

#### Time Assumed for Contribution of Wood or Steel Frame of Floors and Roofs

	Description of Frame	
Type of Assembly	Structural Members	Time, min
Floor(t)	Wood joists, w d l-joists, wood trusses and cold-formed-steel joists spaced ≤ 600 mm o.c.	
FIOGRAD	Open-web ste oists with ceiling supports spaced ≤ 400 mm o.c.	10(2)
Roof Open-web ster oists	Wood joists spiced ≤ 400 mm o.c.	10
	Open-web ste oists with ceiling supports spaced ≤ 400 mm o.c.	
	Wood truss as mblies [metal-plate-connected] spaced ≤ 600 mm o.c.	10
A TOTAL PROPERTY.		5





Table D-2.3.4.-A

Time Assigned to Protective Membranes on Fire-Exposed Side of Wood-Framed and Cold-Formed-Steel-Framed Walls

Description of Finish	Tim	e, min
Description of Fillish	Loadbearing Walls	Non-Loadbearing Walls
11.0 mm Douglas Fir plywood phenolic bonded	-	10(1)
14.0 mm Douglas Fir plywood phenolic bonded	-	15(1)
12.7 mm Type X gypsum board	25(2)	25
15.9 mm Type X gypsum board	40(2)	40(3)
Double 12.7 mm Type X gypsum board <sup>(4)</sup>	50	80

Table D-2.3.4.-B
Time Assigned to Gypsum Board Membranes on Fire-Exposed Side of Floors

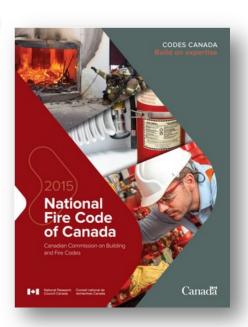
Description of Finish	Resilient Metal Channels(1)	Time, min	
Description of Finish		Floors with Wood or Steel Joists	Floors - th Open-Web Steel Joists
12.7 mm Type X gypsum board	Conned - 400 (0)	25(3)	
15.9 mm Type X gypsum board	Spaced ≤ 400 mm o.c. (2)	40	The state of the s
12.7 mm Type X gypsum board		25(4)	25
15.9 mm Type X gypsum board		40(4)	40
Double 12.7 mm Type X gypsum board	Spaced ≤ 400 mm o.c.(5)	50(3)	-
Double 12.7 mm Type X gypsum board	Spaced at 600 mm o.c.(6)	45(3)	_
Double 15.9 mm Type X gypsum board	Spaced ≤ 600 mm o.c.(6)	60(3)	_





## SUBSECTION 5.6.3. Add't Requirements for C & D Occupancies

- Additional measures during construction
  - Smoking restrictions
  - Signage requirements
  - Disposal specifications
  - Enhanced fencing, boarding and barricades







## SUBSECTION 5.6.3. Firefighting Specifications

- Adequate water supply
  - Once combustible materials on site
- Hydrant Access
  - Clearly marked
  - Clearance not less than 2 m









## SUBSECTION 5.6.3. Construction Access

- Stairway requirements
  - In accordance with NBC dimensions
  - Extended as each floor installed
  - Maintained during demolition







## **A**GENDA



- Wood Construction and the NBC/NFC
- Past Successes (Alternative Solutions)
- Future Opportunities
- Available Tools and Calculations
- Questions





- The two methods of compliance are described in Article 1.2.1.1.
- a.) complying with the applicable Acceptable Solutions in Division B, or
- b.) using Alternative Solutions that will achieve at least the minimum level of performance required by Division B in the areas defined by the objectives and functional statements attributed to the applicable Acceptable Solutions.











- Increasing design complexity
- Is it practical to comply (eg. historic sites ... Province House)



Conventions are being challenged



56 Egress Concepts and Design Approaches

#### Early Scientific Studies of Exit Width

The current 44 in. (1100 mm) minimum exit stair width is intended to support two, 22 in. (550 mm) queues of occupants either standing still (capacity method) or moving down the stair. This also allows counterflow, which occurs when a single queue of occupants moving down are passed by firefighters or other responders moving up. The 22 in. (550 mm) dimension for the width of a person was offered in 1914, originating from soldiers standing in a line [3].

Challenges to the adequacy of the 22 in. (550 mm) dimension include the need to provide for body sway as people move down the stair [8], dimension has been questioned in light of the increasing size and weight of the typical person, especially in the US. The 22 in. dimension refers (or run). Templer [15] adjusted Blondel's forto the width of a person at the shoulders, which is mula for the use of the old (pre French Revoluassumed to be the widest part. Predtechenskii and tion) inch and a modern gait more like 28 in. Milinskii suggest that 4 in. (100 mm) be added to (710 mm) and arrived at the formula, that for low obstructions (like handrails) the additional space is not needed since one's meets the relation  $2R + G = 635 \, \text{mm}$ .

From anthropometric data for modern Americans, the width at the hip is approaching the width at the shoulder, and it seems that this exception may no longer be valid. Thus, with the shoulder width of the 97.5th percentile adult male reaching 20 in. (510 mm) [11] and allowing the 4 in. on each side for handrail and personal (1400 mm), see Fig. 56.1.

Arguably the most comprehensive studies of movement on stairs were conducted by Templer

2017 Templer concluded that the minimum width of an egress stair should be 56 in. (1400 mm).

#### Scientific Studies of Tread Geometry

One of the earliest studies of stair geometry was conducted by a seventeenth century architect in France named Francois Blondel [14]. Blondel was primarily interested in comfort rather than safety and observed that the main stairs of classic cathedrals were comfortable to use and accommodated large numbers of people attending services. He made and the need to allow for some personal space stair height to tread depth was a constant. measurements and found that the ratio of and he related this dimension to the length of the human gait. His formula was 2R + G = 24in., where R is the rise and G is the going  $2R + G = 710 \,\text{mm}$ . The 7 in. rise, 11 in. run stair geometry commonly required in US codes

Templer [15] summarizes a number of research studies of stair geometry and safety. Many such studies were conducted by observing people moving up or down stairs in buildings. Observations in subway or train stations at rush hours provided data for higher population densities. A few studies were conducted in laboratory settings on specially constructed stair space, the new unit of exit width should be 28 in. (700 mm) and the minimum stair width 56 in. conducted several of these studies, including

Most of the studies reviewed concluded that [12], beginning with his doctoral research [13] [15] is a useful metric for the evaluation of stair





#### LIMITATIONS AND CHALLENGES

- Higher level of engineering for design and enforcement teams
- Often greater engineering effort to prepare and review
- Change in occupancy may trigger reanalysis / modifications
- Limited by available time and budgets
- City of Charlottetown Alternate Compliance wrt Wood

Yesterday's Alternative (Performance)



Today's Prescriptive (Code)







## THE STONE JUG – ALTERNATIVE SOLUTION Carbonear, NL (2015)

- New A-2 (Assembly) occupancy
- Part 3; 3 storey w/ basement level
- 3 levels Interconnected "mini atrium"
- Noncombustible construction required
- Wood construction w/ masonry loadbearing
- Analyses included:
  - Review of fire loss statistics
  - Timed egress calculations / fire models









## THE STONE JUG – ALTERNATIVE SOLUTION Carbonear, NL (2015)

- Required upgrades:
  - Increased FRR for floor fire separation
  - New sprinkler system to NFPA 13
  - Occupant loading restrictions
  - Fire safety plan
  - FAS with enhanced smoke detection
- Demonstrated acceptable level of fire safety









### **GAHAN HOUSE – ALTERNATIVE SOLUTION**

### Charlottetown, PE

- Two storey addition
- Four storeys total w/ 280 sq.m
- Heavy Timber w/ Masonry exterior
- NBC requires NC construction
- Alternative Solution
- Did not fit within COC A.C's
- CFD Simulation / TEA



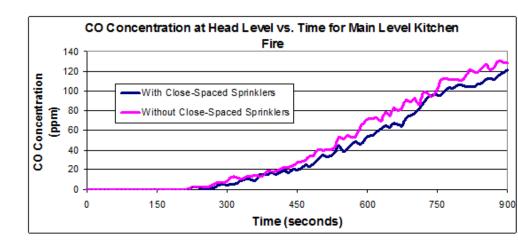






## GAHAN HOUSE – ALTERNATIVE SOLUTION Charlottetown, PE

- TEA Considerations:
  - Temperature/Heat
  - CO Levels
  - Visibility Threshold



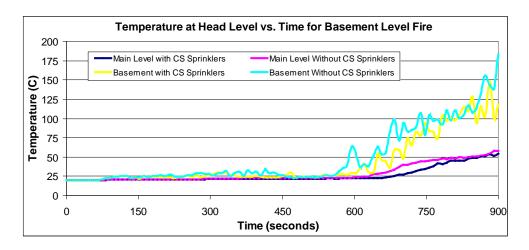
Total Evacuation Times (seconds) for Design Fire Scenario No. 1(Main Level Kitchen)				
	Activation Time	Delay Time	Time for Passage	Total
With Addition	108	180	114	402
Without Addition	108	180	95	383
Added Time with Addition				19 🚁





## GAHAN HOUSE – ALTERNATIVE SOLUTION Charlottetown, PE

- Detection Calculations
  - Smoke detector and sprinkler activation calculations
  - Fire Dynamics Simulator (FDS)







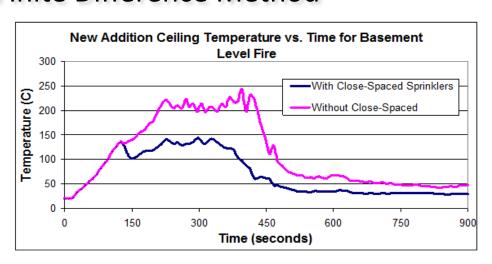


## GAHAN HOUSE – ALTERNATIVE SOLUTION Charlottetown, PE

- Heat Transfer Analysis
  - 1-Dimensional Nodal Finite Difference Method

$$T_i^{p+1} = Fo(T_{i+1}^p + T_{i-1}^p) + (1-2Fo)T_i^p$$

$$Fo = \frac{\alpha \Delta t}{\left(\Delta x^2\right)}$$









## GAHAN HOUSE – ALTERNATIVE SOLUTION Charlottetown, PE

- Design fire scenarios
  - Main level kitchen area
  - Basement level kitchen preparation area

AREA OF FIRE STRUCTURE FIRE DUTCH DOOR KITCHEN WETCHEMICAL SPRINKLER **EXHAUST** SUPPRESSION. SYSTEM ORIGIN LEFT OPEN **ACTIVATES ACTIVATES** ACTIVATE S NO NO NO Kitchen Fire NO YES **EATING & DRINKING** YES YES E STABLISHME NT FIRE YE S



## GAHAN HOUSE – ALTERNATIVE SOLUTION Charlottetown, PE

#### Results

- Ignition temperature not reached
- Tenable conditions during evacuation time (ASET vs RSET)
- Sensitivity analysis

#### **Required Measures**

- Quick response sprinklers
- Water curtain approach at specific locations
- Cover wood assemblies with Type X thermal barrier
- Fire safety plan per NFC 2.8.







## KAYS BUILDING – Charlottetown, PE

- Renovation of Historic 1872 Building
- New mixed Group E / D occupancies
- Wholesaler until 2009 (F-2)
- Four storeys w/ basement level
- 820 sq.m prior to Welsh Owen add't
- Addition of A-2 Assembly on L1











## KAYS BUILDING – Charlottetown, PE

- BSI and NRC methods calc methods
- Estimate FRR for timber structures
- Beams / joists heated on 4 sides
- Fire penetration time @ timber deck

$$R = 2.54fB \left[ 4 - 2 \left( \frac{B}{D} \right) \right]$$
$$t = \zeta \left( \frac{d}{\beta} \right)$$









Structure	Dimension (mm x mm) (w x d)	Heavy Timber <sup>1</sup> (Yes/No)
All Columns	200 x 250	Yes
Beam B1	250 x 280	Yes
Beam B2	150 x 150	No
Beam B3	225 x 225	Yes
Beam B4	250 x 290	Yes
Beam B5	250 x 350	Yes
Beam B6	300 x 290	Yes
Beam B7	240 x 290	Yes
Beam B8	280 x 330	Yes
Beam B9	300 x 355	Yes
Beam B10	300 x 330	Yes
Beam B11	290 x 355	Yes
2" x 8" Joist	38 x 235	No
3" x 8" Joist	64 x 184	No
3" x 12" Joist	64 x 285	No

<sup>1)</sup> As defined by the dimensional requirements of NBC Article 3.1.4.7.

Table A1: Summary of First Floor Timber Structures

Structure	Dimension (mm x mm) (w x d)	Heavy Timber <sup>1</sup> (Yes/No)
All Columns	175 x 175	No
Beam B1 (Built-up)	150 x 150 (top and bottom)	No
Beam B2 (Built-up)	175 x 175 (top and bottom)	No
Beam B3 (Built-up)	150 x 150 (top) 200 x 200 (bottom)	No
Beam B4 (Built-up)	175 x 175 (top) 200 x 200 (bottom)	No
Beam B5 (Built-up)	175 x 175 (top) 200 x 200 (bottom)	No
Beam B6 (Built-up)	190 x 175 (top) 175 x 200 (bottom)	No
Beam B7 (Built-up)	175 x 175 (top) 225 x 200 (bottom)	No
Beam B8 (Built-up)	175 x 200 (top) 150 x 175 (bottom)	No
Beam B9 (Built-up)	200 x 225 (top) 2 – 140 x 140 (bottom)	No
Beam B10 (Built-up)	2 – 25 x 150 (stacked - top) 150 x 200 (middle) 2 – 125 x 125 (bottom)	No
2" x 8" Joist	38 x 235	No
2" x 10" Joist	38 x 184	No

<sup>1)</sup> As defined by the dimensional requirements of NBC Article 3.1.4.7.

**Table A4: Summary of Third Floor Timber Structures** 







## KAYS BUILDING – ALTERNATIVE SOLUTION Charlottetown, PE

- Beams and columns estimated > 45 min FRR w/ LVL covering
- Acceptable level of fire and life safety
- Check against NFPA 101A FSES

#### Required Measures

- Enhanced early warning (smoke detection)
- Enhanced sprinkler system
- Fire safety plan
- Occupancy type restriction / re-evaluation with A-2







## YELLOWBELLY – ALTERNATIVE SOLUTION St. John's, NL

- Four storey with Basement
- New Group A-2 (Assembly)
- Originally built in 1725 ... one of North Americas oldest
- 1846 Fire and survived Great Fire of 1892









# YELLOWBELLY – ALTERNATIVE SOLUTION St. John's, NL

- Performance-based solution
- Computational Fluid Dynamics (CFD) simulations









## YELLOWBELLY – ALTERNATIVE SOLUTION St. John's, NL

Critical section of wooden beams (similar for columns)

$$kZ\frac{BD^2}{6} = \propto \frac{bd^2}{6}$$

Fire endurance time

$$t_{f} = \begin{cases} 2.54ZB \left( 4 - \frac{2B}{D} \right) 4 - sided\ exposure \\ 2.54ZB \left( 4 - \frac{B}{D} \right) 3 - sided\ exposure \end{cases}$$

$$Z - \left\{ 0.7 + \frac{0.3}{R} \right\} R < 0.5$$

$$R \ge 0.5$$









#### YELLOWBELLY – ALTERNATIVE SOLUTION

St. John's, NL

Timed Exit Analysis

ThirdLevel	301	Lounge Area No 1	144.5	1.2	121
	302	Lounge Area No 2	40.4	1.2	34
	303	Bar	27.0	1.2	23
£		A	vailable Exi	Subtotal t Capacity	178 235

- Worst-case Travel Distance = 16 m
- Travel Speed = 1.19 m/s 50% to be conservative = 0.595 m/s
- Cueing Time = 112 s
- Travel + Cueing = 139 s
- Activate Smoke Detection = 12 s
- Total time = 151 s
- +++ Pre-movement times

- Considerations:
  - Temperature/Heat Exposure
  - Visibility Threshold



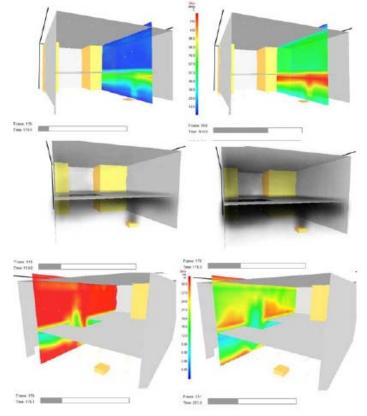




#### YELLOWBELLY – ALTERNATIVE SOLUTION

St. John's, NL

- Worst case design fires
  - Third storey dining area
  - Second storey kitchen
- Fire growth scenarios by broad range of fuels
- Sensitivity Analysis









## YELLOWBELLY – ALTERNATIVE SOLUTION St. John's, NL

- Proposed conditions will not compromise safety
- Required measures:
  - Passive protection for columns
  - Limitations on occupant loading
  - Fire safety plan







## PAST SUCCESSES (OF OTHERS)

# 2010 VANCOUVER OLYMPICS The Richmond Olympic Oval

- Example of an alternative solution
- Conventional light frame lumber
- Sprinkler lines
- Mineral wool insulation
- FDS Software









### PAST SUCCESSES (OF OTHERS)

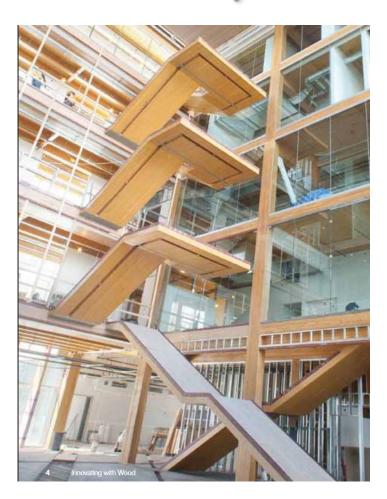
#### **UBC Campus**

WOOD works!

- Earth Sciences Building (GHL)
- BioEnergy Research (LMDG)









#### **AGENDA**



- Wood Construction and the NBC/NFC
- Past Successes (Alternative Solutions)
- Future Opportunities
- Available Tools and Calculations
- Questions





### **FUTURE OPPORTUNITIES**

#### **MULTI STOREY WOOD CONSTRUCTION**

- Six storeys is new to the NBC coming for many provinces
- Eight storeys for next step for NBC??? 13 being evaluated
- Fire safety of timber lining and cladding materials
- Fire resistance of pre-stressed timber frames and walls









### **FUTURE OPPORTUNITIES**

#### The Evolution of Building Codes

- Fire Statistics / Loss History effecting change
- Increased reliance and acceptance of alternative solutions
- Ever changing technologies
- Less Federal \$\$ for research and development
- Leveraging <u>industry partners</u> to drive innovation





## PERFORMANCE OF CLT ASSEMBLIES IN FIRE

Noureddine Benichou, Ph.D., National Research Council Canada Christian Dagenais, Eng., M.Sc., FPInnovations

May 28, 2015

NRC-CNRC







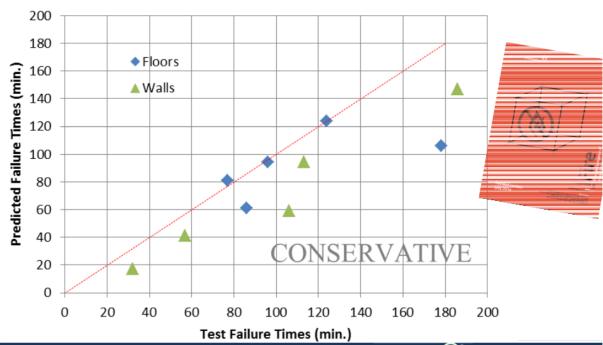






#### **CLT FR Calculation Methods**















Home » Publications » Research Papers / Journals

#### **Research Papers / Journals**

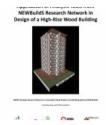
May 27, 2015

#### "Checker Building" Report now available for download.

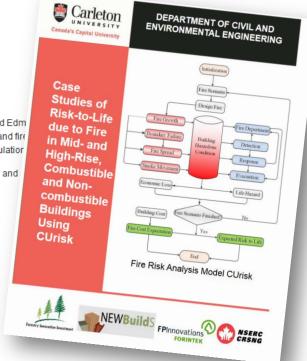
NEWBuildS presented the "CHECKER BUILDING" – a 20-storey conceptual wood building in Vancouver, BC, Montreal, QC and Edm report includes the design results on the architecture, building envelope, lateral and gravity load resisting systems, fire risks and fire conceptual 20-storey wood-hybrid building. This 124 page report has comprehensive drawings, illustrations and design calculation

This report is of great interest to designers, specifiers, wood products producers, researchers, government regulatory bodies and to take the leader role in the design of tall wood buildings in Canada.

#### DOWNLOAD: CHECKER Building Report



May 27, 2015





### **FUTURE OPPORTUNITIES**

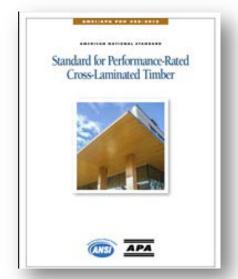
CONTEMPORARY RESEARCH CLT – Cross Laminated Timber



ANSI/APA PRG 320-2011 Standard for Performance-Rated Cross-

**Laminated Timber** 









# FIRE RATING CALCULATIONS Structural resistance of CLT floor assembly 90 minutes of exposure

Factored Load

$$w_{f(fire)} = D_{L} + 0.5L_{L} = (5.1) + 0.5 \cdot (20.5) = 15.4 \text{ kN/m}^{2}$$

$$m_{f(fire)} = \frac{w_{f(fire)} 1^{2}}{8} = \frac{(15.4) \cdot (4730)^{2}}{8} = 43.1 \text{ kNm/m}$$

- Char depth
  - Charring rate = 0.65 mm/min  $d_{char} = \beta t = (0.65 \text{ mm/min}) \cdot (90 \text{ min}) = 58.5 \text{ mm}$
  - Remaining cross-section





$$D_{char} = D - d_{char} = (38 \text{ mm} \times 5) - 58.5 = 131.5 \text{ mm}$$



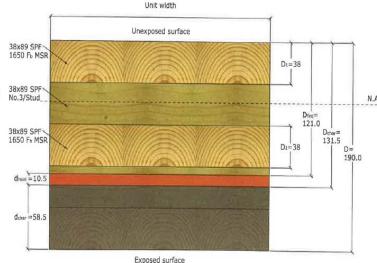
# FIRE RATING CALCULATIONS Structural resistance of CLT floor assembly after 90 minutes of exposure

- Effective residual cross-section
  - Heated zone for floors = 10.5 mm
  - Remaining cross-section

$$D_{\text{fire}} = D_{\text{char}} - d_{\text{heat}} = 131.5 \text{ mm} - 10 \text{ mm} = 121.5 \text{ mm}$$

Location of neutral axis

$$\overline{y} = \frac{\sum_{i} \widetilde{y}_{i} D_{i}}{\sum D_{i}} = \frac{\left[ \left( \frac{38}{2} \right) \cdot \left( 38 \right) + \left( \left( \frac{38}{2} \right) + \left( 2 \times 38 \right) \right) \cdot \left( 38 \right) \right]}{\left( 38 + 38 \right)} = 57.0 \text{ mm}$$





# FIRE RATING CALCULATIONS Structural resistance of CLT floor assembly after 90 minutes of exposure

#### Moment of inertia

$$I = \sum_{i} \frac{B D_{i}^{3}}{12} + \sum_{i} B D_{i} d_{i}^{2} = \left[ \frac{(1000) \cdot (38)^{3}}{12} + \frac{(1000) \cdot (38)^{3}}{12} \right] + \left[ (1000) \cdot (38) \cdot \left( 57.0 - \frac{38}{2} \right)^{2} + (1000) \cdot (38) \cdot \left( 38 + 38 + \frac{38}{2} - 57.0 \right)^{2} \right] = 119 \times 10^{6} \text{ mm}^{4}$$

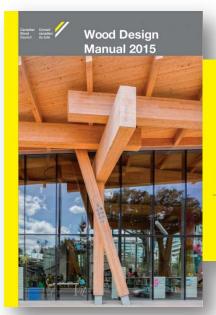
#### Moment resistance

$$M_r = \varphi F_b S K_{Zb} K_L = (1.0) \cdot (27.5) \cdot (2.09 \times 10^6) \cdot (1.0) \cdot (1.0) = 57.5 \text{ kNm/m}$$





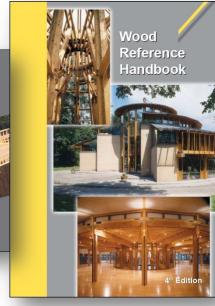
## ONLINE RESOURCES Canadian Wood Council technical manuals











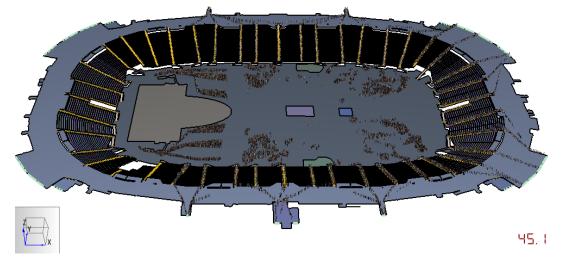


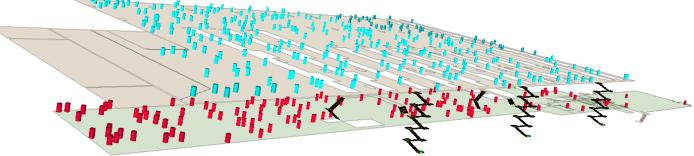


## PATHFINDER Movement Simulation



- Evacuation
- Animated results

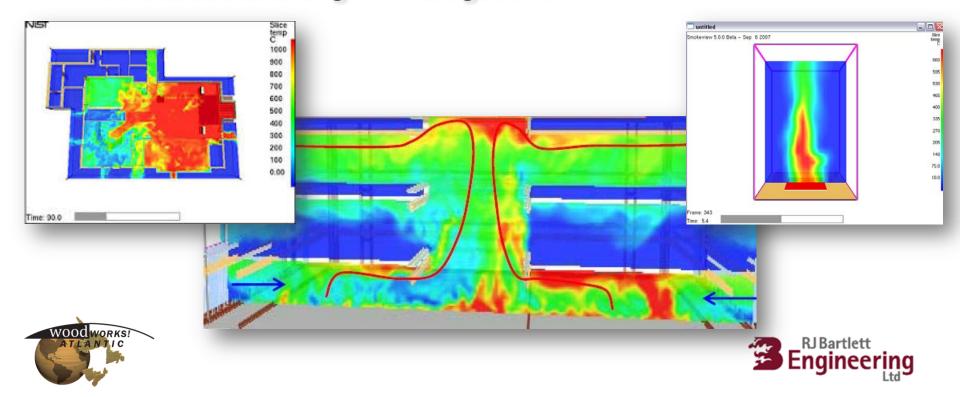






#### FIRE DYNAMICS SIMULATOR (FDS)

- Computational fluid dynamics of wood construction
- Fire cause and origin investigations



# ARCHAIC HANDBOOK Test FRR in existing construction

Table 1.3.2 Wood Frame Walls, 4" (100 mm) to less than 6" (150 mm) thick

ltem	Thick-	Construction Details	Performance		Reference Number			Notes	Rec
Code	ness		Load	Time	Pre BMS 92	BMS	Post- BMS 92		Hours
W-4-W-1	4"	2" x 4" stud wall; 3/16" CAB; no insulation; design A	35 min	10 min	-	-	4	1–10	1/6
W-4-W-2	4 1/8"	2" x 4" stud wall; 3/16" CAB; no insulation; design A	38 min	9 min	-	-	4	1–10	1/6
W-4-W-3	4 3/4"	2" x 4" stud wall; 3/16" CAB and 3/8" gypsum board face (both sides); design B	62 min	64 min	-	-	4	1–10	1
W-5-W-4	5"	2" x 4" stud wall; 3/16" CAB and 1/2" gypsum board face (both sides), design B	79 min	>90 min	-	-	4	1–10	1







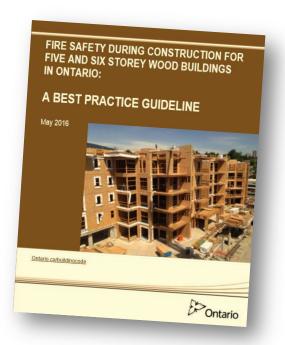








# ONTARIO BEST PRACTICE GUIDELINE Canadian Wood Council Collaboration with Ontario Ministries and Stakeholders

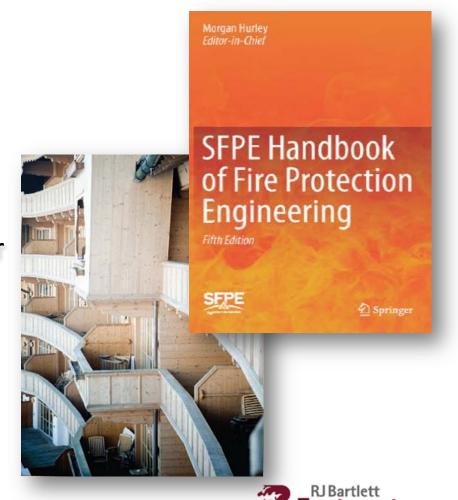






#### GUIDANCE FROM OTHER COUNTRIES SFPE Handbook

- Guidelines in Europe
- Fire resistance of heavy timber





Space for Educational Complexes

Fires per million square feet per year

#### Guidance from other countries

**NFPA 557** 

Sprinklers / FAS present =

Fires per year

CONSTRUCTION TYPE / FRR less significant
Millions of square feet in buildings with at least 1000 ft<sup>2</sup>

Fires per thousand buildings per year

25.0

0.91

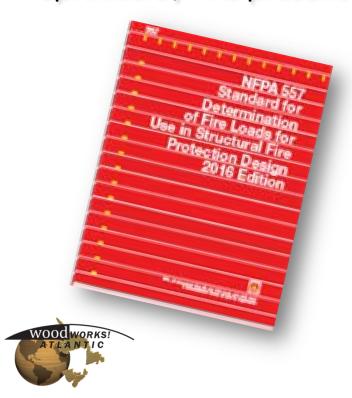
Table D.5(a) Rate of Fires (per year) Relative to Numbers of Buildings and Square Feet of Floor



	No Spri	inklers	Sprinklers	Sprinklers Present		
Type of Construction	No Detectors	Detectors Present	No Detectors	Detectors Present		
Fire resistive	7% 12,140	3% 9,878	4% 1,017	2% 4,293		
Protected, noncombustible	7% 5,544	4% 4,753	2% 689	3% 2,826		
Unprotected, noncombustible	9% 4,040	4% 3,071	1% 251	2% 652		
Protected, ordinary	8% 8,215	4% 6,025	5% 737	3% 2,786		
Unprotected, ordinary	16% 6,169	8% 3,962	4% 308	5% 858		
Protected, wood frame	18% 2,794	7% 1.595	5% 263	2% 647		
Unprotected, wood frame	30% 5,108	13% 1,692	11% 179	3% 313		

Sources: NFPA analysis of NFIRS; NFPA survey; Energy Information Administration Commercial Buildings Energy Consumption Surveys, building characteristics tables.

Note: These are 1989–1998 fires reported to U.S. municipal fire departments and so exclude fires reported only to federal or state agencies or industrial fire brigades. These years are used because they are the latest for the type of construction that is included in the coded elements. All estimates are based on at least 200 reported fires (raw, not projected estimates) in the 10 years with the indicated data known. Buildings and floor space are estimated from 1992, 1995, and 1999 surveys, using linear interpolation and extrapolation for years before or between the three years when surveys were taken, resulting in a final formula of {(7 × 1992 estimate) + [1.5 × (1995 estimate + 1999 estimate)]/10.



## QUESTIONS





